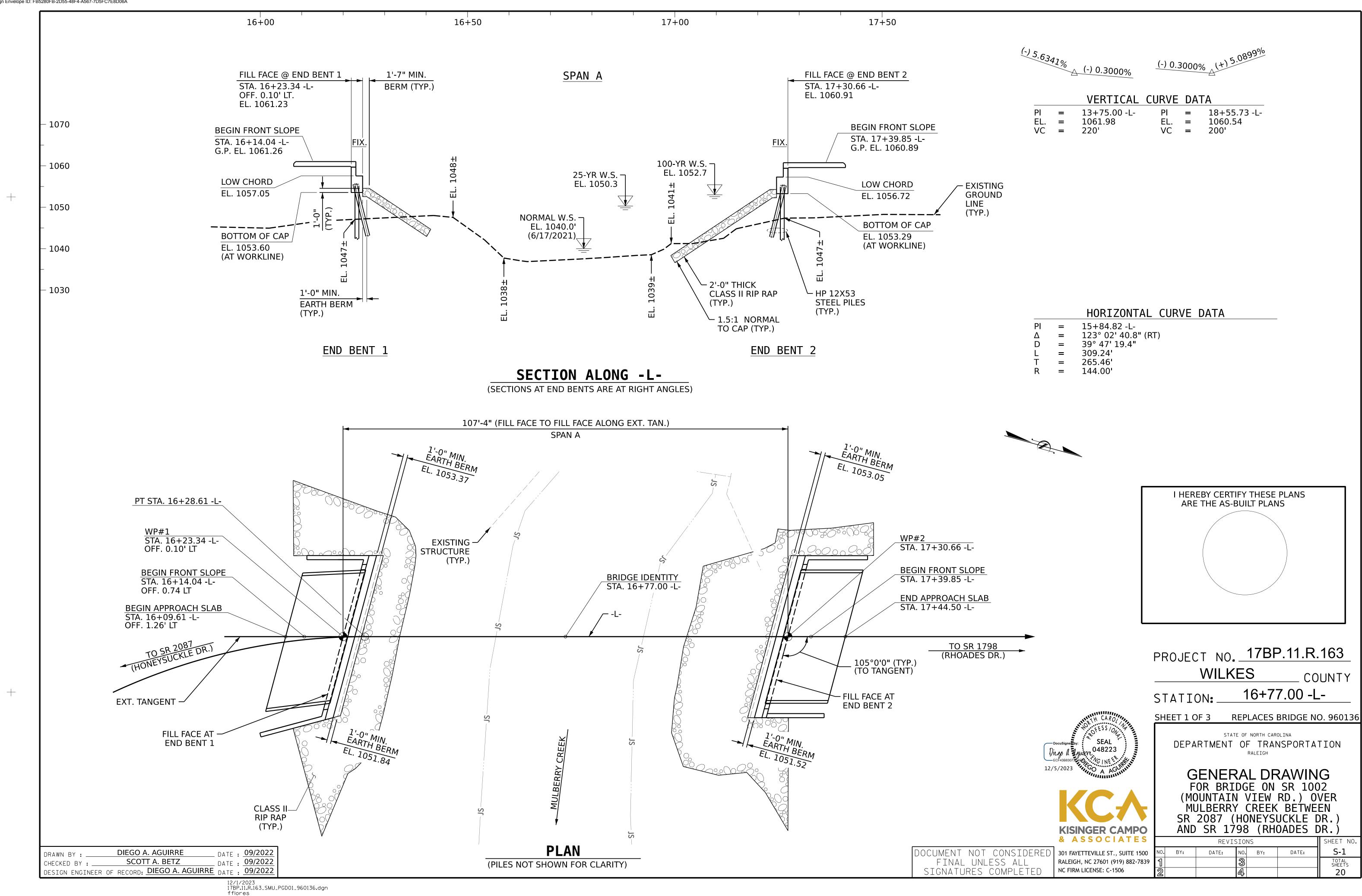


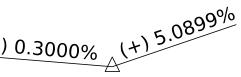
STATE STATE	PROJECT REFERENCE NO.	SHEET	TOTAL SHEETS			
N.C. 17B	<u> </u>	20				
STATE PROJ. NO.	F. A. PROJ. NO.	DESCRIPT	ION			
17BP.11.PE.163	N⁄A	P.E	P.E.			
17BP.11.ROW.163	N⁄A	R∕W, UTI	LITIES			
17BP.11.R.163	N/A	CONST.				
L						



Prepared i	n the Office of:
	801 FAYETTEVILLE ST., SUITE 1500 RALEIGH, NC 27601 (919) 882-7839 NC FIRM LICENSE: C-1506
2024 STANDARD SPECIFICATIONS	
<i>TING DATE :</i> SEE ROADWAY PLANS	DIEGO A. AGUIRRE, Ph.D., P.E. PROJECT ENGINEER FIDEL L. FLORES, E.I. PROJECT DESIGN ENGINEER



(-) 5.6341%	
11%	(-) 0.3000



		VERTICAL	CURVE	DAT	Ā
PI EL. VC	= = =	13+75.00 -L- 1061.98 220'		=	18+55.73 -L- 1060.54 200'

ΡI	=	15+84.82 -L-
Δ	=	123° 02' 40.8" (RT)
D	=	39° 47' 19.4"
L	=	309.24'
Т	=	265.46'
R	=	144.00'

## SUMMARY OF PILE INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

					Driven Piles			Predrilling for Piles*			Drilled-In Piles		
End Bent/ Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5")	Factored Resistance per Pile TONS	Pile Cut-Off (Top of Pile) Elevation FT	Estimated Pile Length per Pile FT	Scour Critical Elevation FT	Min Pile Tip (Tip No Higher Than) Elev FT	Required Driving Resistance (RDR)** per Pile TONS	Total Pile Redrives Quantity EACH	Predrilling Length per Pile Lin FT	Predrilling Elevation (Elev Not To Predrill Below) FT	Maximum Predrilling Dia INCHES	Pile Excavation (Bottom of Hole) Elev FT	Pile Exc Not In Soil per Pile Lin FT	Pile Exc In Soil per Pile Lin FT
End Bent 1, Piles 1-7	120	1055.60	25			200							
End Bent 2, Piles 1-7	120	1055.29	25			200							
							-						

\*Predrilling for Piles is required for end bents/bents with a predrilling length and at the Contractor's option for end bents/bents with predrilling information but no predrilling length.

 $**RDR = \frac{Factored Resistance + Factored Downdrag Load + Factored Dead Load}{Powndrag Resistance + Factored Downdrag Resistance + Nominal Scour Resistance + Scour Resistance + Cour Resistanc$ 

## PILE DESIGN INFORMATION

(Blank entries indicate item is not applicable to structure)

End Bent/ Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5")	Factored Axial Load per Pile TONS	Factored Downdrag Load per Pile TONS	Factored Dead Load* per Pile TONS	Dynamic Resistance Factor	Nominal Downdrag Resistance per Pile TONS	Nominal Scour Resistance per Pile TONS	Scour Resistance Factor (Default = 1.00)
End Bent 1, Piles 1-7	118			0.60			1.00
End Bent 2, Piles 1-7	118			0.60			1.00

\*Factored Dead Load is factored weight of pile above the ground line.

NOTES:

- 3. The Engineer will determine the need for PDA Testing and Pipe Pile Plates when PDAs or plates may be required.
- 4. For piles, see pile provision and section 450 of the standard specifications.
- does not release the contractor from providing driving equipment in accordance with subarticle 450-3(D)(2) of the Standard Specifications.

Nominal Scour Resistance

1. The Pile Foundation Tables are based on the bridge substructure design and foundation recommendations sealed by a North Carolina Professional Engineer (Cheng Wang and 048123) on 01-09-2023. 2. Total Pile Driving Equipment Setup quantity (not shown in Pile Foundation Tables) equals the number of driven piles, i.e., the number of piles with a Required Driving Resistance.

5. It have been estimated that a hammer with an equivalent rated energy in the range of 20,000 to 25,000 ft-lbs per blow will be required to drive piles at End Bent No.1 and No.2. This estimated energy range

Pile D End Bent/ Bent No End Bent 1, Piles 1-7 End Bent 2, Piles 1-7

\*EST = Pile order lengths from estimated pile lengths; PDA = Pile order lengths based on PDA testing. For groups of end bents/bents with pile order lengths based on PDA testing, the first end bent/bent no. listed for each group is the representative end bent/bent with the PDA.

End Pont/	Dino Dilo	S			
End Bent/ Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5")	Pipe Pile Plates Required? YES or MAYBE	Pipe Pile Cutting Shoes Required? YES	Pipe Pile Conical Points Required? YES	H-Pile Points Required? YES	Steel Pile Tips Required? YES
End Bent 1, Piles 1-7				Yes	
End Bent 2, Piles 1-7				Yes	
TOTAL QTY:				14	

# SUMMARY OF PDA/PILE ORDER LENGTHS

(Blank entries indicate item is not applicable to structure)

Driving Analyz	er (PDA)		Pile Order Lengths				
PDA Testing Required? YES or MAYBE	PDA Test Pile Length FT EACH 30 30		End Bent/ Bent No(s)	Pile Order Length Basis* EST or PDA			
YES		1					

### SUMMARY OF PILE ACCESSORIES

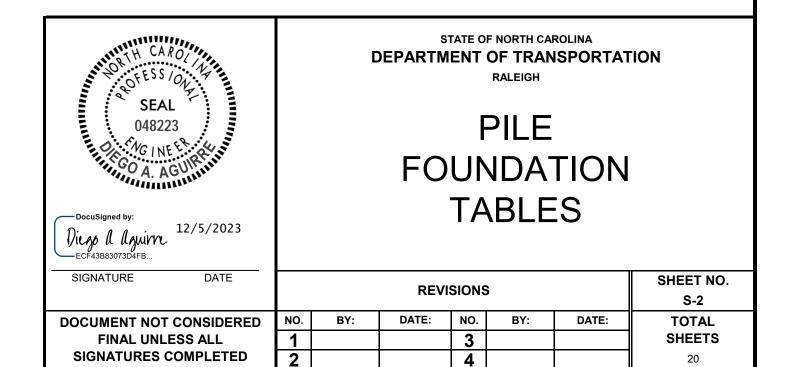
(Blank entries indicate item is not applicable to structure)

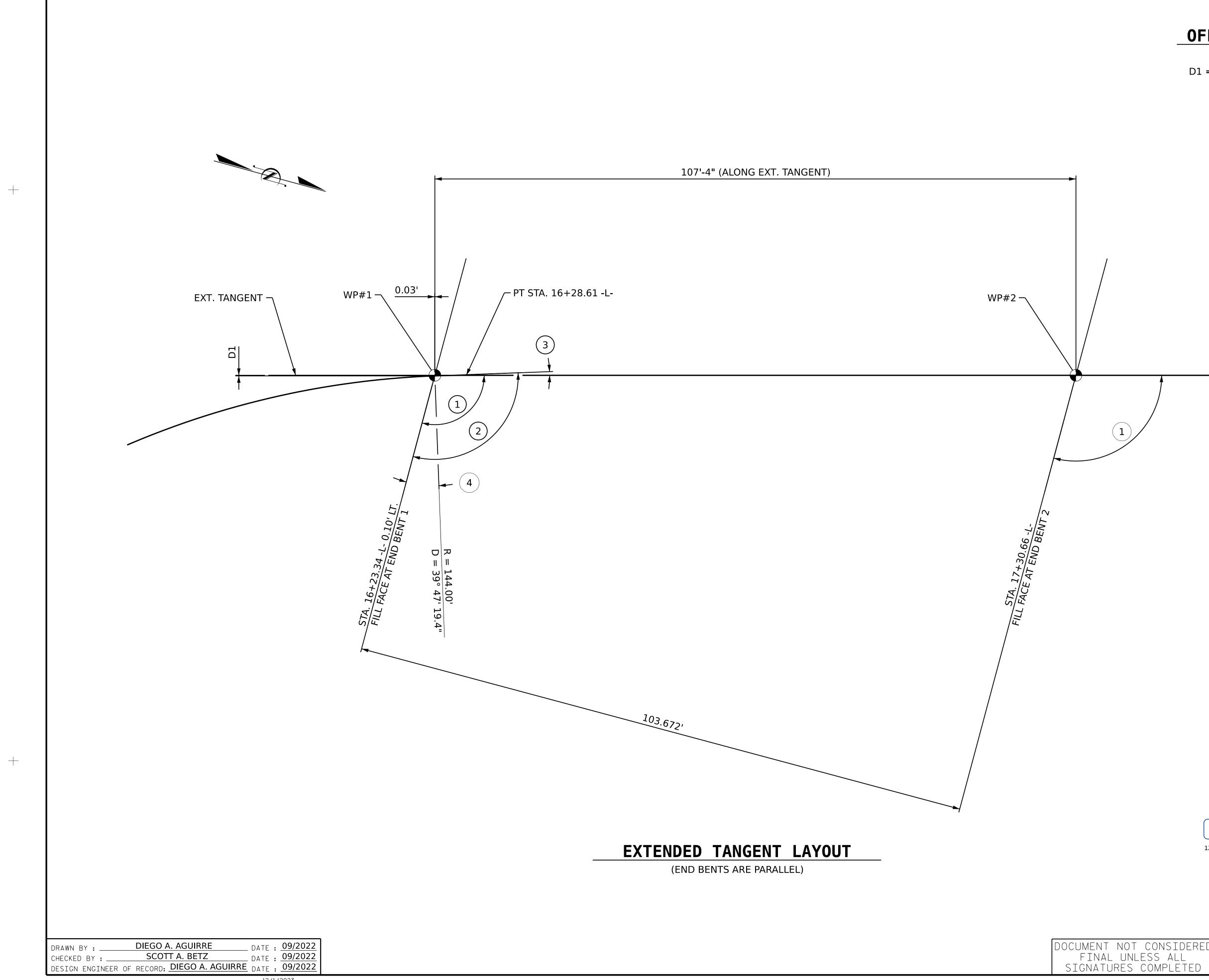
PROJECT NO. <u>17BP.11.R.163</u>

Wilkes

\_COUNTY

STATION: <u>16+77.00 -L-</u>





12/1/2023 17BP.11.R.163\_SMU\_PGD02\_960136.dgn fflores

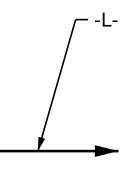
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# **OFFSETS**

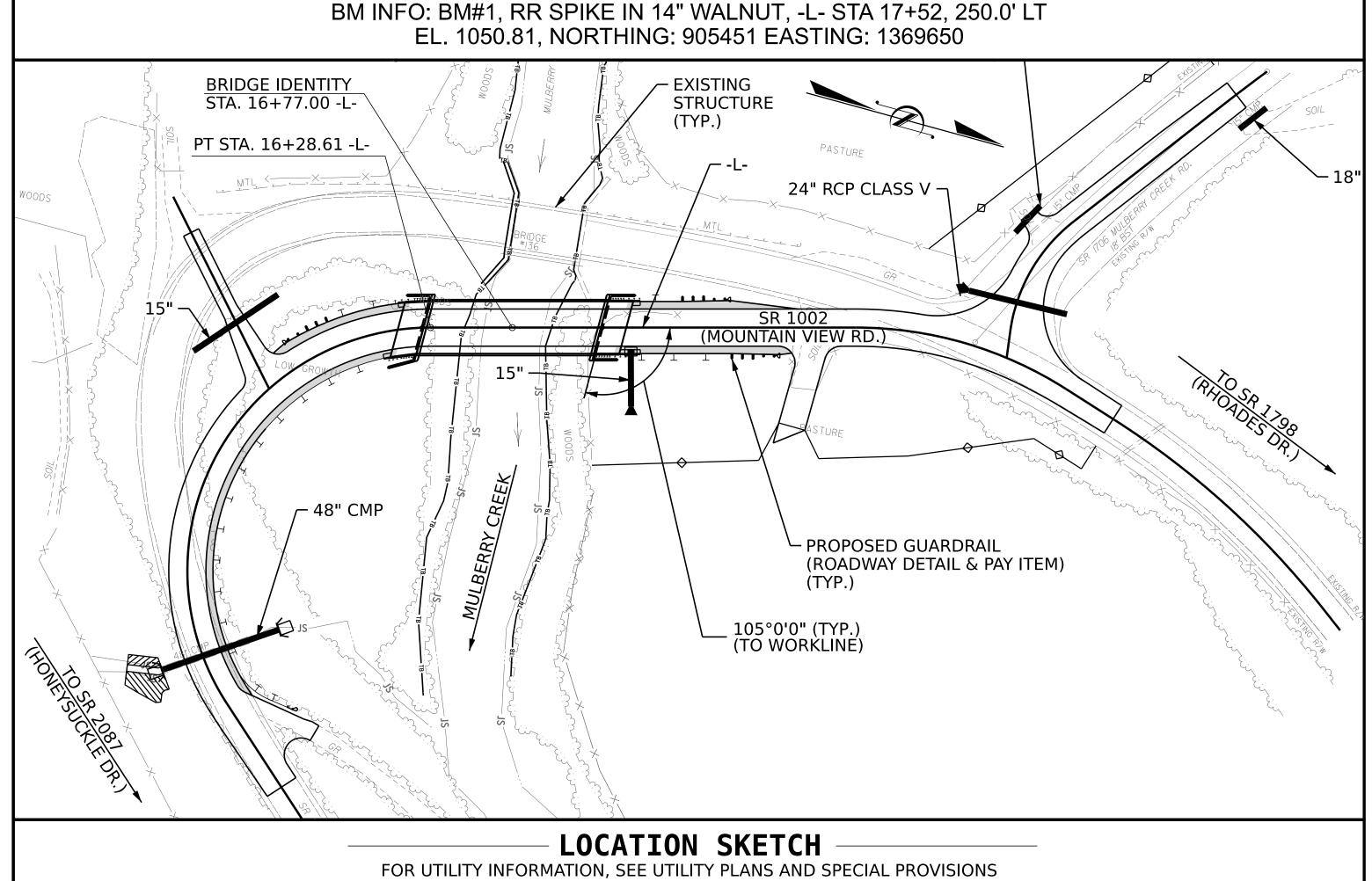
D1 = 0.10' LT.

# ANGLES

- 1 105° 0' 0" TO WORKLINE
- 2 107° 6' 29" TO TANGENT
- 3 2° 6' 29"
- 4 17° 5' 47" TO RADIAL LINE



PROJECT NO. 17BP.11.R.1									
	WILKES COUNTY								
	STATI	DN:	16+7	7.00 -L					
TH CAROL	SHEET 2 C	)F 3 R	EPLACES	BRIDGE N	D. 960136				
Docusigne by: SEAL Dicas A Hawry 048223 ECF43B830754ES, NG INE 4	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH								
12/5/2023 THIGO A AGUINT	G		RAL DF	RAWIN	G				
KCA	(MC ML SR	UNTAIN JLBERRY 2087 (	I VIEW CREEK HONEYS		VER EN DR.)				
KISINGER CAMPO	AND SR 1798 (RHOADES DR.)								
= D 301 FAYETTEVILLE ST., SUITE 1500	NO. BY:	REVIS	NO. BY:	DATE:	SHEET NO. <b>S-3</b>				
RALEIGH, NC 27601 (919) 882-7839 NC FIRM LICENSE: C-1506	1		3 4		TOTAL SHEETS 20				



TOTAL BILL OF MATERIAL											
	REMOVAL OF EXISTING STRUCTURE STA. 16+77.00 -L-	ASBESTOS ASSESSMENT	CLASS A CONCRETE (BRIDGE)	BRIDGE APPROACH SLABS	REINFORCING STEEL (BRIDGE)	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	HP 12	2 X 53 STEEL PILES	STEEL PILE POINTS	DYNAMIC PILE TESTING	
	LUMP SUM	LUMP SUM	CU. YD.	LUMP SUM	LBS.	EA.	No.	LIN. FT.	EA.	EA.	
SUPERSTRUCTURE											
END BENT 1			30.3		5085	7	7	175	*	*	
END BENT 2			29.7		4982	7	7	175	*	*	
TOTAL	LUMP SUM	LUMP SUM	60.00	LUMP SUM	10067	14	14	350	14	1	

TOTAL BILL OF MATERIAL CONT										
	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS		3'-3" PRESTRESSED CRETE BOX BEAM				
	LIN. FT.	TONS	SQ. YDS.	LUMP SUM	No.	LIN. FT.				
SUPERSTRUCTURE	210.00				11	1155.00				
END BENT 1		232	365							
END BENT 2		275	373							
TOTAL	210.00	507	738	LUMP SUM	11	1155.00				

\* SEE "PILE FOUNDATION TABLES" SHEET FOR QUANTITIES

DRAWN	BY :	DIE	GO A. AGUIR	RE	DATE :	09/2022
СНЕСКЕ	D BY :	0	SCOTT A. BET	Ζ	DATE :	09/2022
DESIGN	I ENGINEER	OF REC	CORD: DIEGO A	A. AGUIRRE	DATE :	09/2022

+

12/1/2023 17BP.11.R.163\_SMU\_PGD03\_960136.dgn fflores

# **GENERAL NOTES**

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE **REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN** SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

THIS STUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES".

REMOVAL OF EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENT DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE CONSISTING OF ONE 45'-6" SPAN PRESTRESSED CONCRETE CORED SLABS WITH A CLEAR ROADWAY OF 29'-3" ON PRECAST CONCRETE CAPS WITH CONCRETE ENCASED STEEL PILES AND TIMBER ABUTMENTS LOCATED APPROXIMATELY 25' UPSTREAM FROM PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

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INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 16 + 77.00 -L-".

FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY. SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND **RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.** 

# HYDRAULIC DATA

DESIGN DISCHARGE FREQUENCY OF DESIGN FLOOD DESIGN HIGH WATER ELEVATION DRAINAGE AREA BASE DISCHARGE (Q100) BASE HIGH WATER ELEVATION

5400 CFS 25 YRS. 1050.3 FT. 39.1 SQ. MI. 7500 CFS 1052.7'

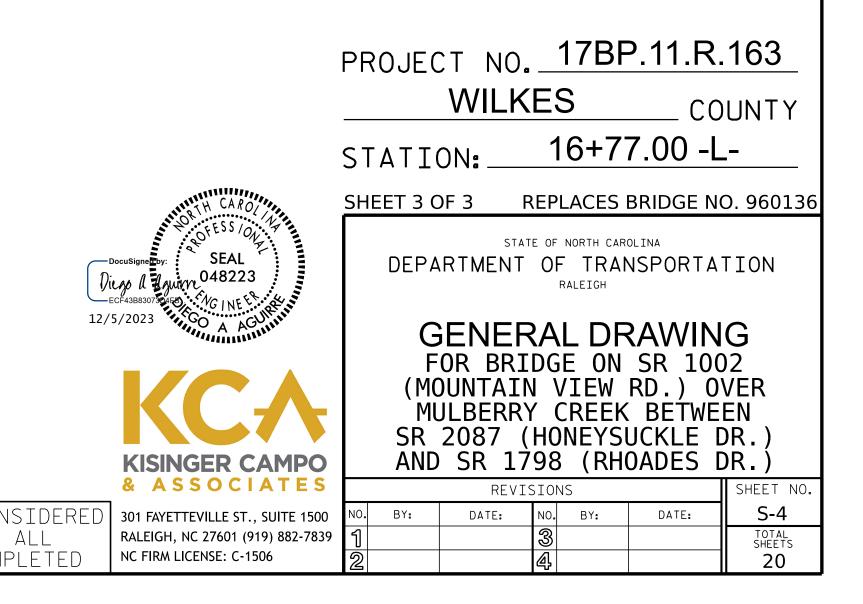
# **OVERTOPPING FLOOD DATA**

OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING FLOOD OVERTOPPING FLOOD ELEVATION

18300 CFS 500+ YRS. 1061.2'

SAG STA.

17+67.00 -L-



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											STRENG	THILIMI	T STATE						SER	<b>VICE III L</b>	.IMIT ST	ATE		
										MOMENT					SHEAR						MOMENT			1
I EVFI		Vehicle	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS (YLL)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE LOAD FACTORS (YLL)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
DESIGN	-	HL-93 (INV)	N/A		1.60		1.75	0.209	2.15	A	EL	52.760	0.571	1.60	Α	EL	4.140	0.8	0.209	1.79	А	EL	50.690	<b> </b>
LOAD		HL-93 (OP)	N/A		2.07		1.35	0.209	2.79	A	EL	52.760	0.571	2.07	Α	EL	4.140	N/A						
RATING		HS-20 (INV)	36.000	2>	2.16	77.8	1.75	0.209	3.02	А	EL	52.760	0.571	2.16	Α	EL	4.140	0.8	0.209	2.52	А	EL	50.690	
NATINO		HS-20 (OP)	36.000		2.81	101.2	1.35	0.209	3.92	А	EL	52.760	0.571	2.81	Α	EL	4.140	N/A						
		SNSH	13.500		5.37	72.5	1.4	0.209	9.01	А	EL	52.760	0.571	5.37	Α	EL	4.140	0.8	0.209	6.02	А	EL	51.730	
	ш	SNGARBS2	20.000		4.34	86.8	1.4	0.209	6.49	А	EL	49.660	0.571	4.61	Α	EL	51.730	0.8	0.209	4.34	А	EL	50.690	
	HICLE	SNAGRIS2	22.000		4.05	89.1	1.4	0.209	6.06	А	EL	49.660	0.571	4.30	Α	EL	4.140	0.8	0.209	4.05	А	EL	50.690	
	SV VEI	SNCOTTS3	27.250		2.99	81.5	1.4	0.209	4.48	А	EL	51.730	0.571	3.32	А	EL	99.310	0.8	0.209	2.99	А	EL	51.730	
	;) NGLE	SNAGGRS4	34.925		2.45	85.6	1.4	0.209	3.66	А	EL	52.760	0.571	2.70	А	EL	99.310	0.8	0.209	2.45	А	EL	50.690	
	SING	SNS5A	35.550		2.40	85.3	1.4	0.209	3.59	А	EL	51.730	0.571	2.71	А	EL	4.140	0.8	0.209	2.40	А	EL	51.730	
LEGAL	S	SNS6A	39.950		2.18	87.1	1.4	0.209	3.26	А	EL	52.760	0.571	2.45	Α	EL	99.310	0.8	0.209	2.18	А	EL	51.730	
LOAD		SNS7B	42.000		2.07	86.9	1.4	0.209	3.10	А	EL	51.730	0.571	2.38	Α	EL	4.140	0.8	0.209	2.07	А	EL	51.730	
RATING		TNAGRIT3	33.000		2.65	87.5	1.4	0.209	3.97	А	EL	51.730	0.571	2.93	Α	EL	4.140	0.8	0.209	2.65	А	EL	51.730	
		TNT4A	33.075		2.65	87.6	1.4	0.209	3.97	А	EL	52.760	0.571	2.88	Α	EL	4.140	0.8	0.209	2.65	А	EL	50.690	
	TRACTOR AILER (TTS	TNT6A	41.600		2.15	89.4	1.4	0.209	3.22	А	EL	52.760	0.571	2.47	A	EL	4.140	0.8	0.209	2.15	А	EL	50.690	
	TRA	TNT7A	42.000		2.15	90.3	1.4	0.209	3.22	А	EL	51.730	0.571	2.44	А	EL	4.140	0.8	0.209	2.15	А	EL	51.730	
	RA RA	TNT7B	42.000		2.19	92.0	1.4	0.209	3.28	А	EL	52.760	0.571	2.34	А	EL	4.140	0.8	0.209	2.19	А	EL	50.690	
	TRUCK MI- TR/	TNAGRIT4	43.000		2.11	90.7	1.4	0.209	3.16	А	EL	51.730	0.571	2.27	А	EL	4.140	0.8	0.209	2.11	А	EL	51.730	
	SEMI	TNAGT5A	45.000		2.00	90.0	1.4	0.209	2.99	А	EL	52.760	0.571	2.23	А	EL	4.140	0.8	0.209	2.00	А	EL	50.690	
	S	TNAGT5B	45.000	3	1.98	89.1	1.4	0.209	2.97	А	EL	49.660	0.571	2.17	А	EL	4.140	0.8	0.209	1.98	А	EL	50.690	
EMERG	ENCY	EV2	28.750		3.05	87.7	1.3	0.209	4.92	А	EL	49.660	0.571	3.52	А	EL	4.140	0.8	0.209	3.05	А	EL	50.690	
VEHICL	E (EV)	EV3	43.000	$\langle 4 \rangle$	2.01	86.4	1.3	0.209	3.25	А	EL	52.760	0.571	2.37	А	EL	4.140	0.8	0.209	2.01	А	EL	50.690	

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For Span " A "

DESIGN ENGINEER OF RECORD:	
DIEGO A. AGUIRRE	DATE : 01/2023
ASSEMBLED BY : FIDEL L. FLORES CHECKED BY : SCOTT A. BETZ	DATE : 01/2023 DATE : 01/2023

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## LOAD FACTORS:

DESIGN	LIMIT STATE	γDC	γDW
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

## NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

## $\langle \# \rangle$ CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

 $\langle 3 \rangle$  LEGAL LOAD RATING \* \*

\*\* 4 EMERGENCY VEHICLE LOAD RATING

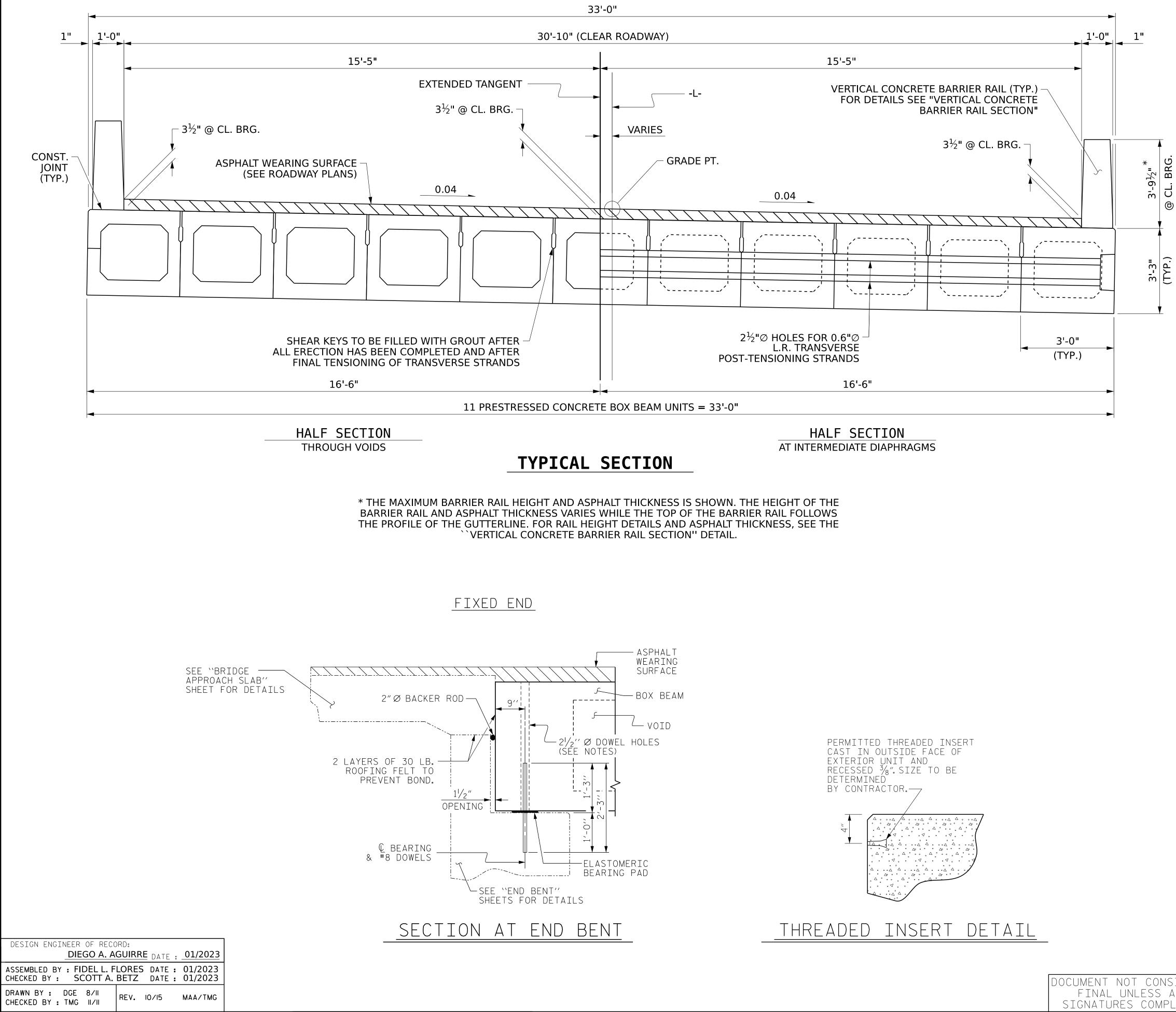
\* \* SEE CHART FOR VEHICLE TYPE

## GIRDER LOCATION

- I INTERIOR GIRDER
- EL EXTERIOR LEFT GIRDER
- ER- EXTERIOR RIGHT GIRDER

	PROJEC STATIC	WILK	ES	P.11.R. C0 7.00 -L	UNTY
DocuSigner by: SEAL Dicas & Agurry 048223 ECF43B8307304FE)	LF	RTMENT S RFR S 05' BC	RALEIGH	NSPORTA RD RY FC AM UN	R
KISINGER CAMPO	(N	ON-INT	ERSTATI	E TRAFF	-
& ASSOCIATES		REVIS		2.1.75	SHEET NO.
ERED 301 FAYETTEVILLE ST., SUITE 1500 RALEIGH, NC 27601 (919) 882-7839	NO. BY:	DATE:	NO. BY:	DATE:	S-5
ED NC FIRM LICENSE: C-1506	1 2		৩ 4		TOTAL SHEETS 20

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# NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE  $2^{1}/_{2}$  " Ø dowel holes at fixed ends of box beam sections SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6,500 PSI.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS,  $\frac{1}{2}$ " in depth, shall be tooled in all exposed faces of the barrier rail and in accordance with article 825-10(b) of the standard SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

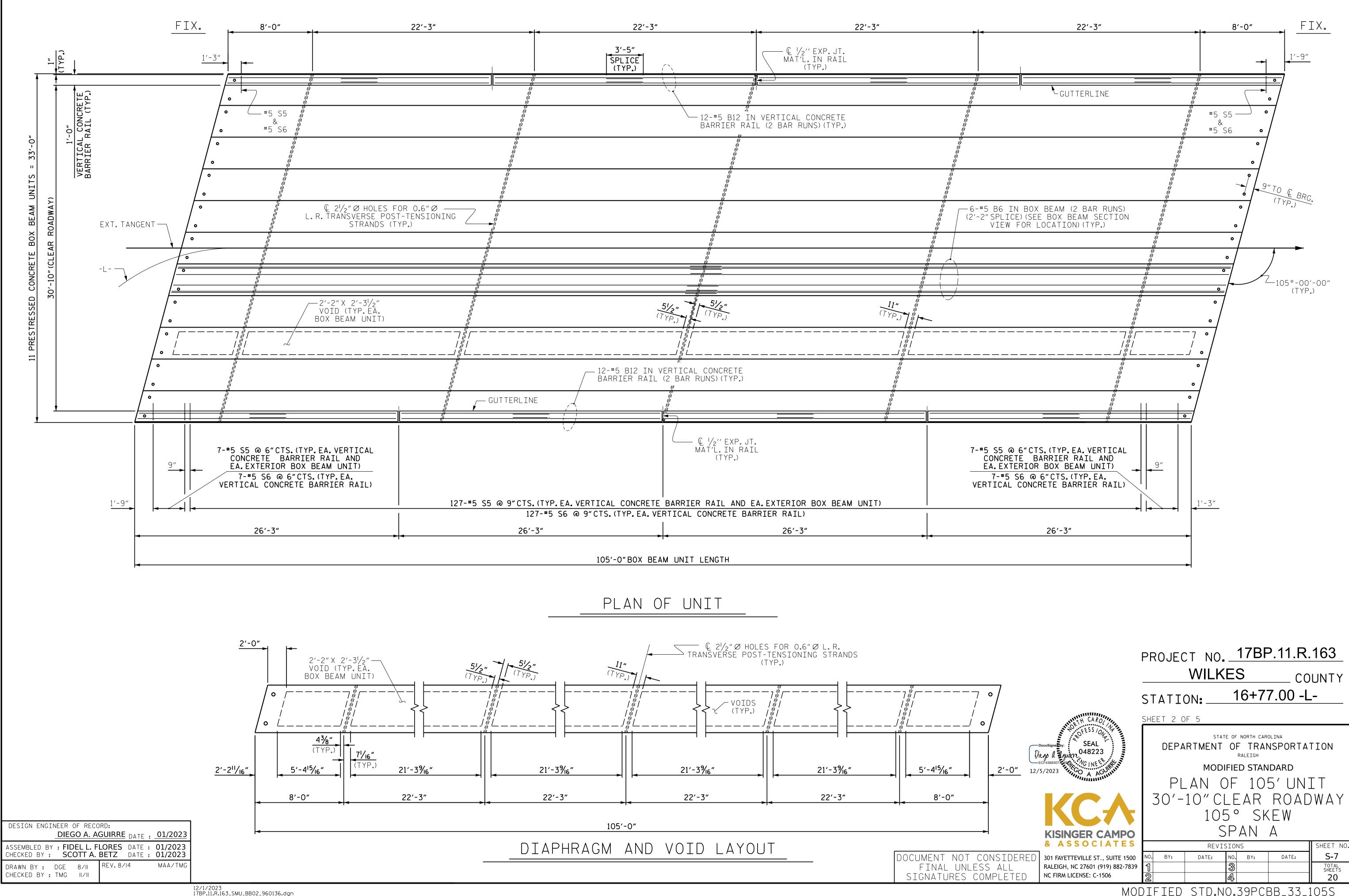
THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

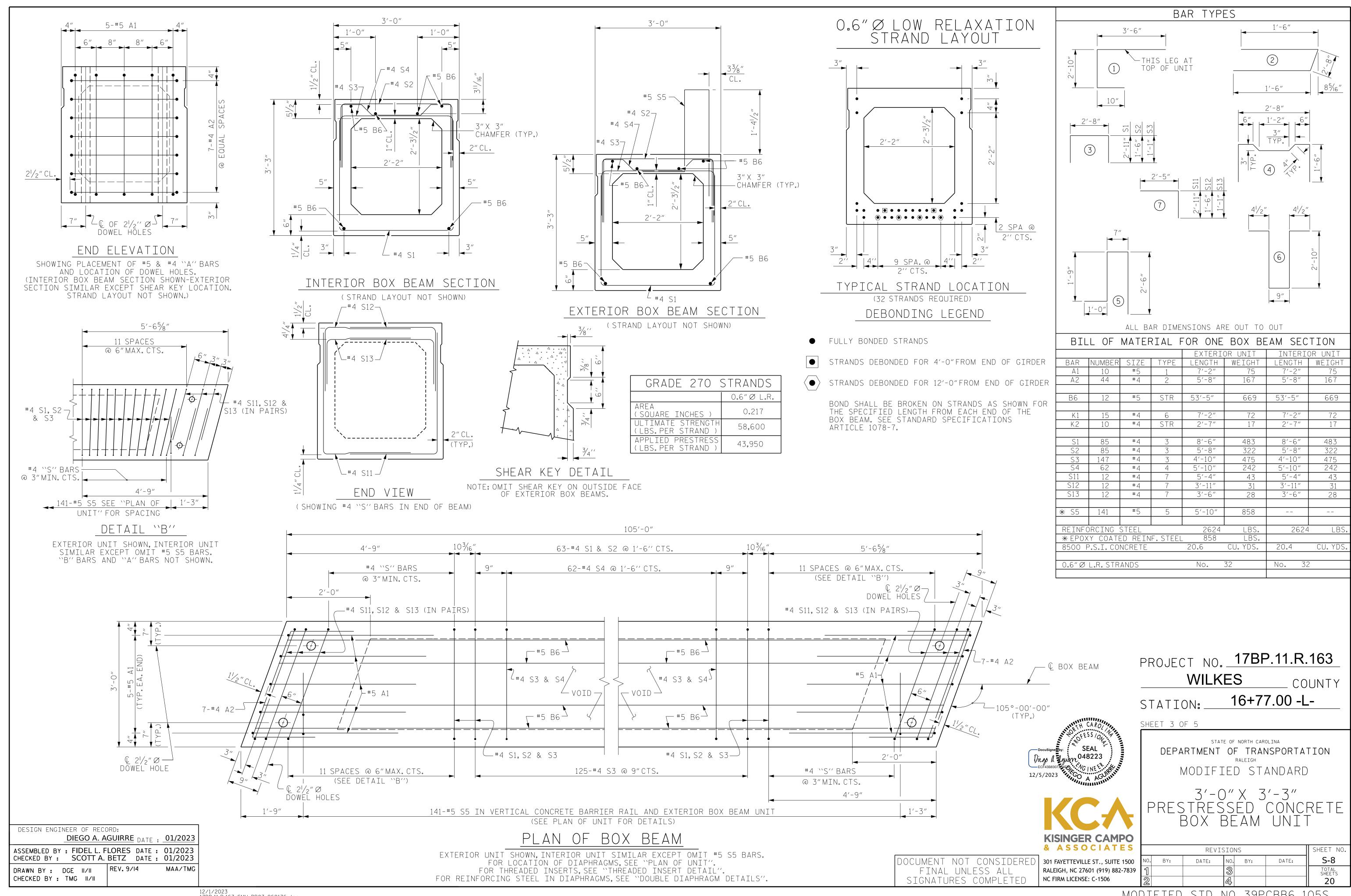
THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK. THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

	PROJECT NO. <u>178P.11.R.163</u>
	WILKES COUNTY
	STATION: <u>16+77.00 -L-</u>
ATH CAROLINA	SHEET 1 OF 5
DocuSigne by: SEAL Dicas A Hawry 048223 ECF43B8307544EQ. No LILE	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
12/5/2023 A AGUILIN	MODIFIED STANDARD
KCA	3'-O"X 3'-3" Prestressed concrete Box beam unit
KISINGER CAMPO	REVISIONS SHEET NO.
RED 301 FAYETTEVILLE ST., SUITE 1500	NO. BY: DATE: NO. BY: DATE: S-6
RALEIGH, NC 27601 (919) 882-7839	1 3 TOTAL SHEETS   2 4 20
	ODIFIED STD.NO.39PCBB1_33

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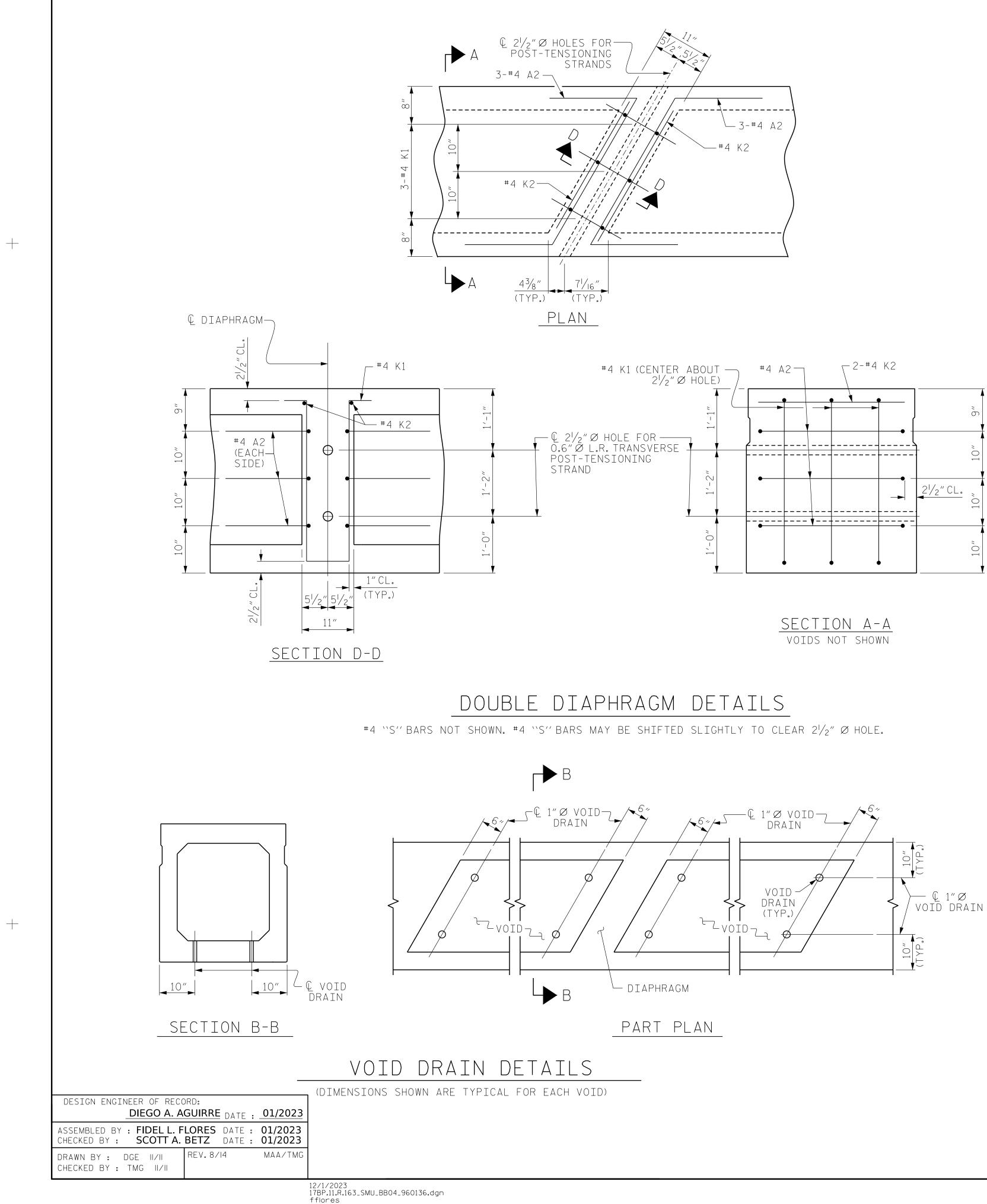


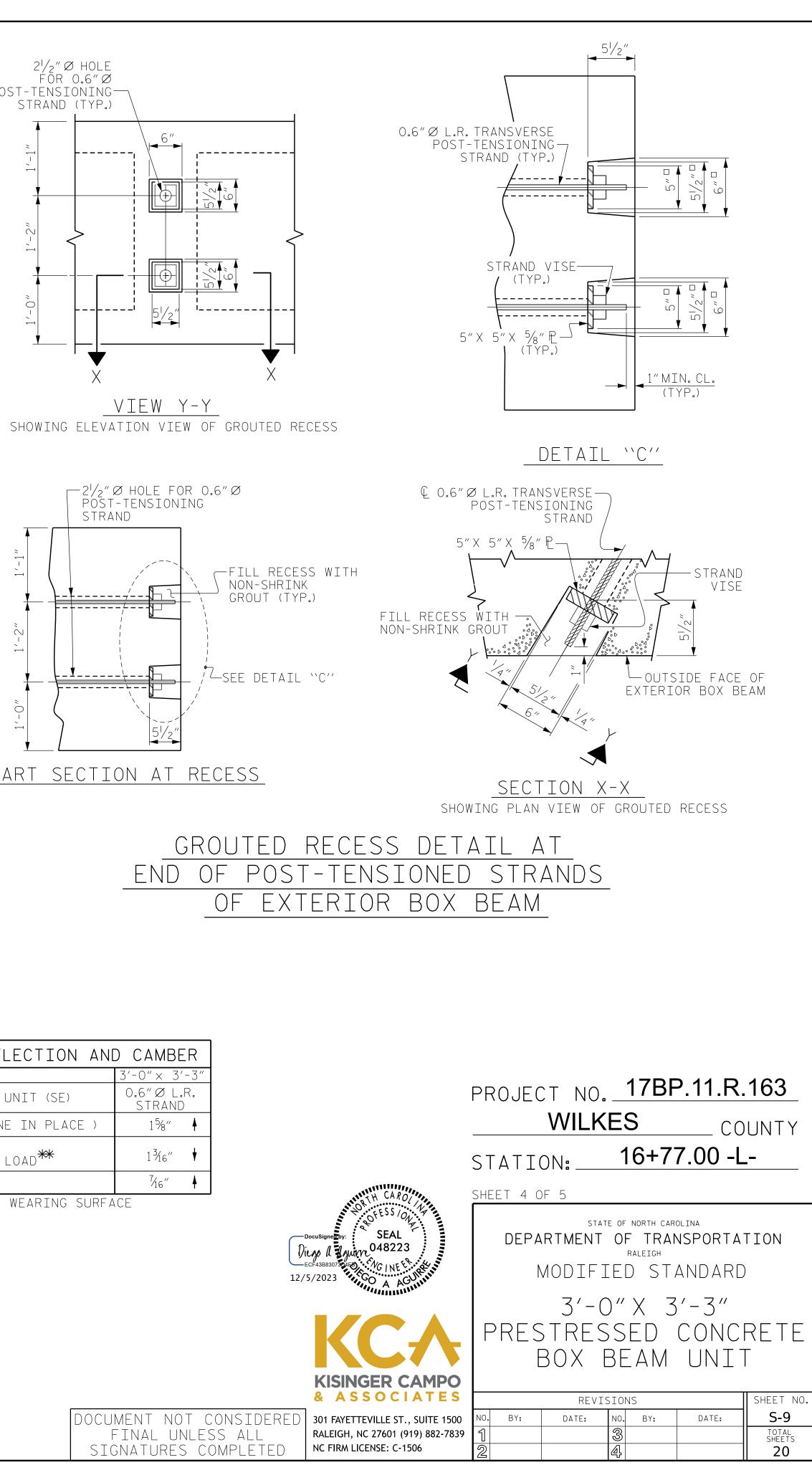
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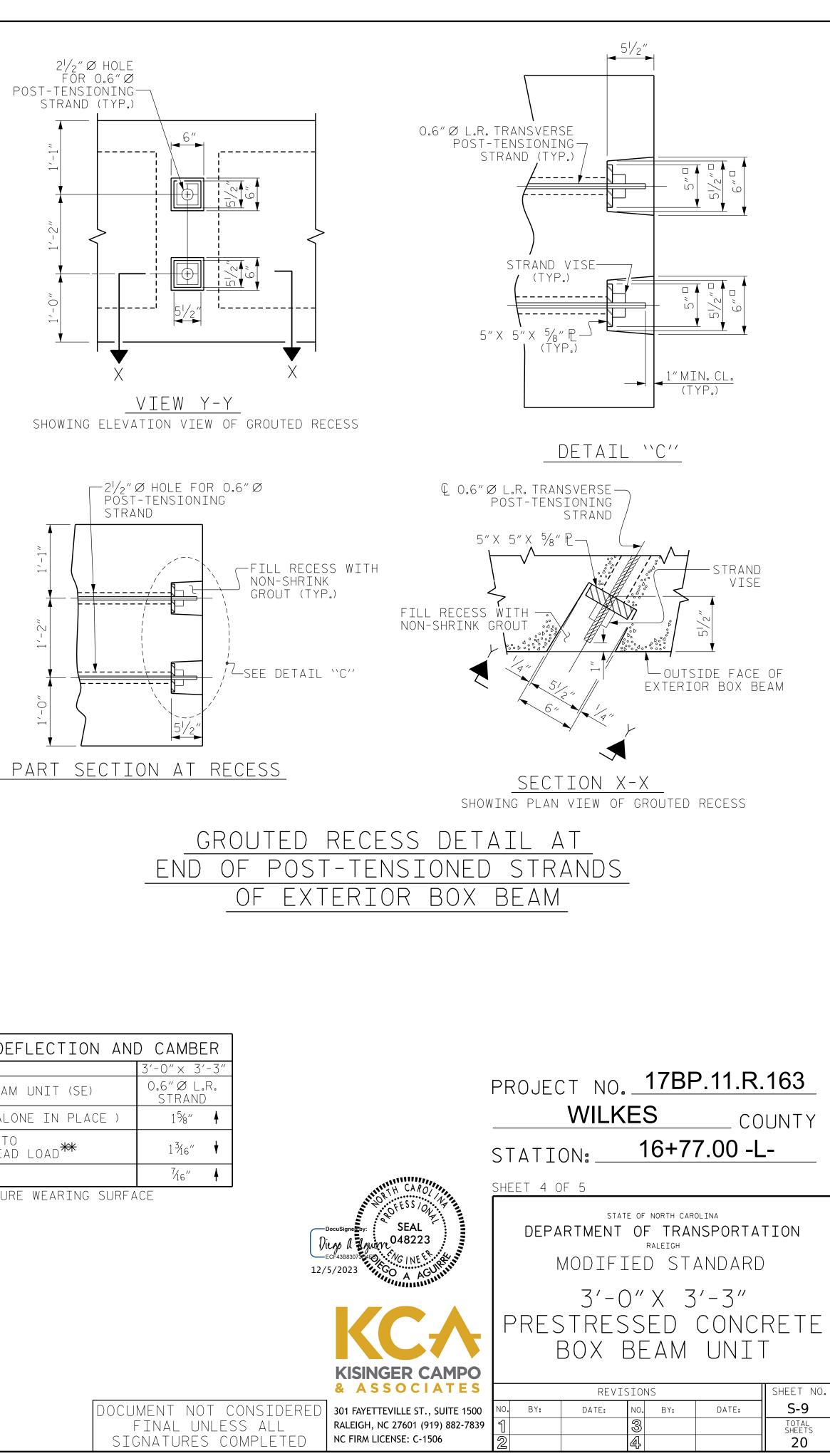


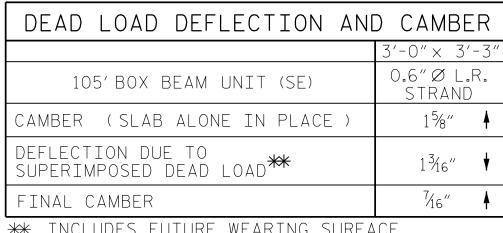
<sup>12/1/2023</sup> 17BP.11.R.163\_SMU\_BB03\_960136.dgn fflores

MODIFIED STD. NO. 39PCBB6\_105S





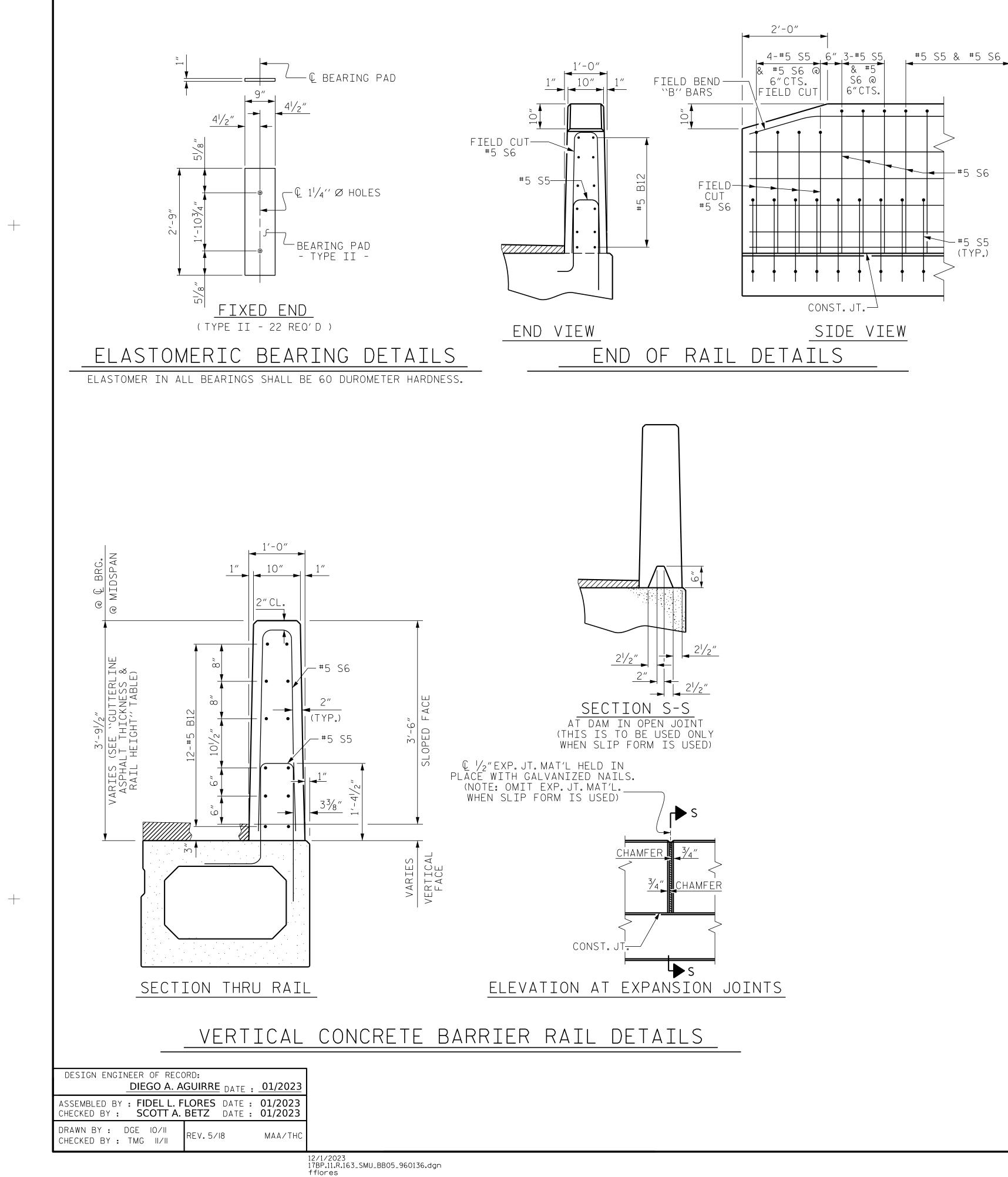




\*\* INCLUDES FUTURE WEARING SURFACE

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MODIFIED STD.NO.39PCBB7\_105S



GUTTERLINE ASP	HALT THI	CKNESS	5 & RAI	L HEIGHT	
	ASPHALT OV @	/ERLAY TH MID-SPAN	ICKNESS	RAIL HEIGHT @ MID-SPAN	6″ ••
105' UNITS		3¼6″		3'-9¼6''	
B	OX BEA	M UN	NITS F	REQUIRED	7 <sup>3</sup> / <sub>4</sub> "
		NUMBER	LENGTH	TOTAL LENGTH	
E	XTERIOR B.B.	2	105'-0"	210'-0"	BAR TYPE
I	NTERIOR B.B.	9	105'-0"	945′-0″	
	TOTAL	11		1155'-0"	BAR DIMENSIONS ARE OUT TO OUT

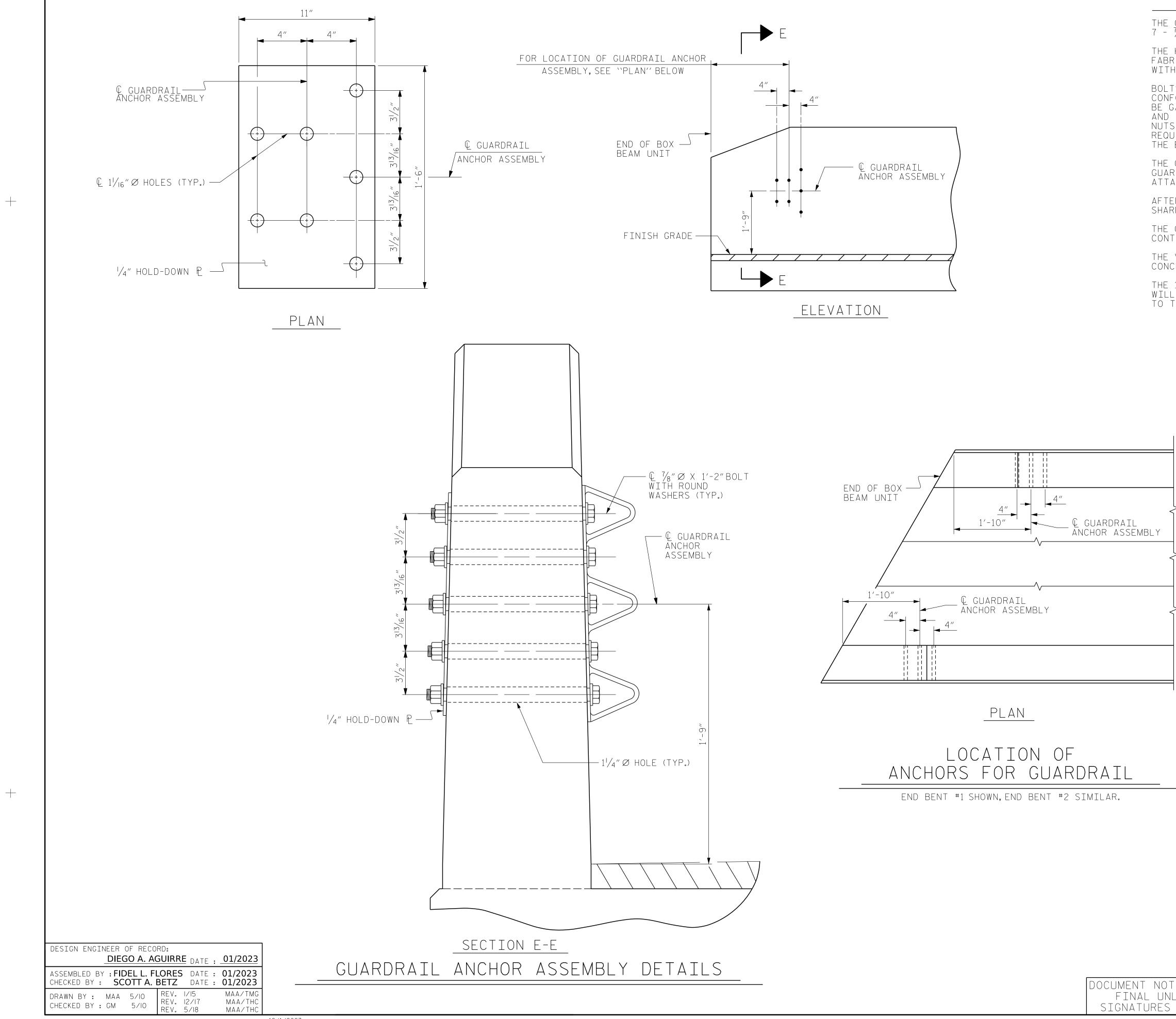
ASPH	HALT THI	CKNESS	& RAIL	_ HEIGHT	
		VERLAY TH MID-SPAN	ICKNESS	RAIL HEIGHT @ MID-SPAN	6″
		3¼6″		3′-9¼6′′	
В	OX BEA	AM UN	NITS F	REQUIRED	7 <sup>3</sup> / <sub>4</sub> "
		NUMBER	LENGTH	TOTAL LENGTH	$\bigcirc 1$
ΕX	TERIOR B.B.	2	105'-0"	210'-0"	
IN	NTERIOR B.B.	9	105'-0"	945′-0″	<u>BAR TYPE</u>
	TOTAL	11		1155'-0"	BAR DIMENSIONS ARE OUT TO OUT
· · · · ·				1	

BI	LL OF MATERIAL FOR VERTICAL CONCRE	ΤΕ Β	ARR	IER F	RAIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT
	105' UNIT				
<b>★</b> B12	192	#5	STR	14'-10"	2970
<b>米</b> S6	282	#5	1	7'-2"	2108
* EPOX	 (Y COATED REINFORCING STEEL		LBS.		F070
					5078
CLASS AA CONCRETE			CU.YDS.		27.2
TOTAL	TOTAL VERTICAL CONCRETE BARRIER RAIL				210.00

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	PROJECT NO. 17BP.11.R.163
	WILKES COUNTY
	STATION: <u>16+77.00 -L-</u>
ATH CAROLINA	SHEET 5 OF 5
DocuSigne by: SEAL Dicas & January 048223 ECF43B830750450 12/5/2023	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH MODIFIED STANDARD 3'-0"X 3'-3" PRESTRESSED CONCRETE
	BOX BEAM UNIT
<b>KISINGER CAMPO</b>	
& ASSOCIATES	REVISIONS SHEET NO.
RED 301 FAYETTEVILLE ST., SUITE 1500	NO. BY: DATE: NO. BY: DATE: S-10
RALEIGH, NC 27601 (919) 882-7839	1 3 TOTAL SHEETS   2 4 20

MODIFIED STD.NO.39PCBB8\_75&105S



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WITH AASHTO M111. THE ENGINEER.)

THE 1  $\frac{1}{4}$  "  $\emptyset$  holes shall be formed or drilled with a core bit. Impact tools WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

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## NOTES

THE <u>G</u>UARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 -  $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE

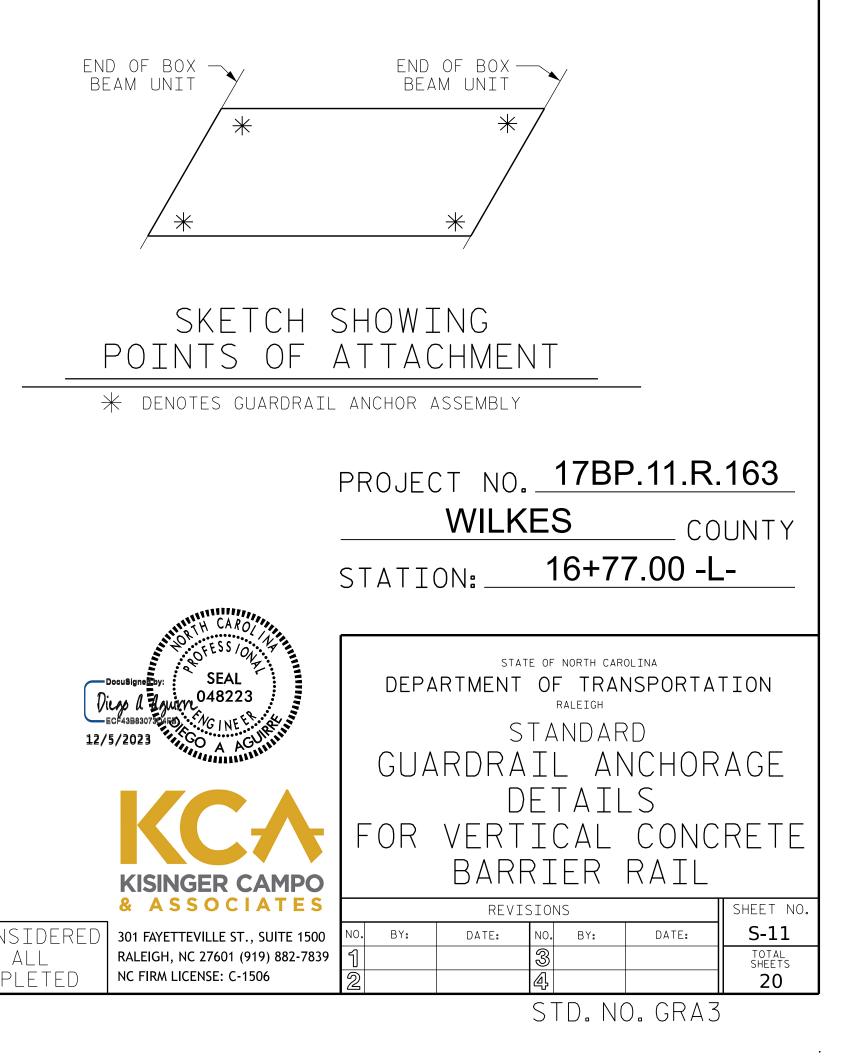
BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/811 Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

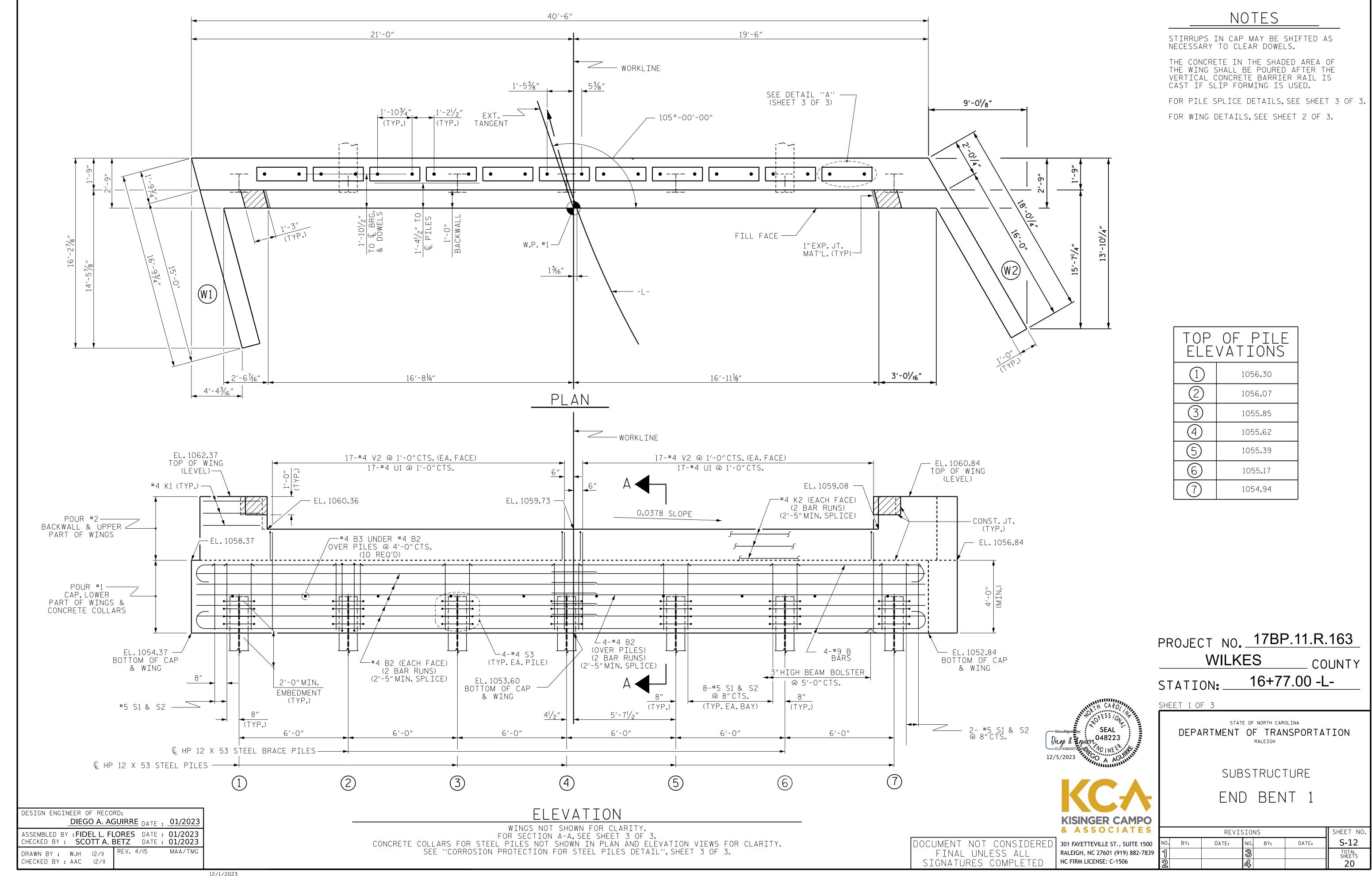
AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

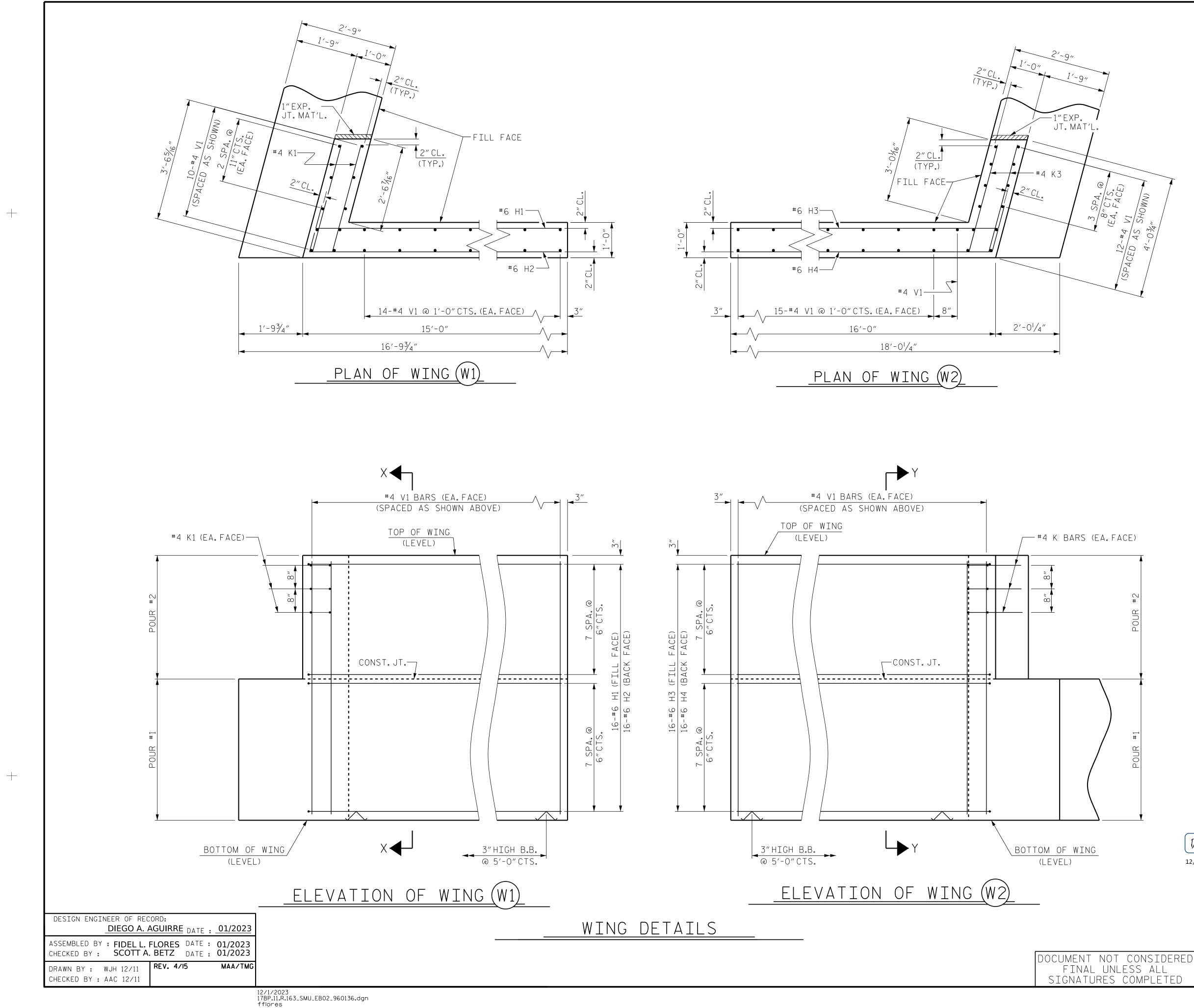
THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.



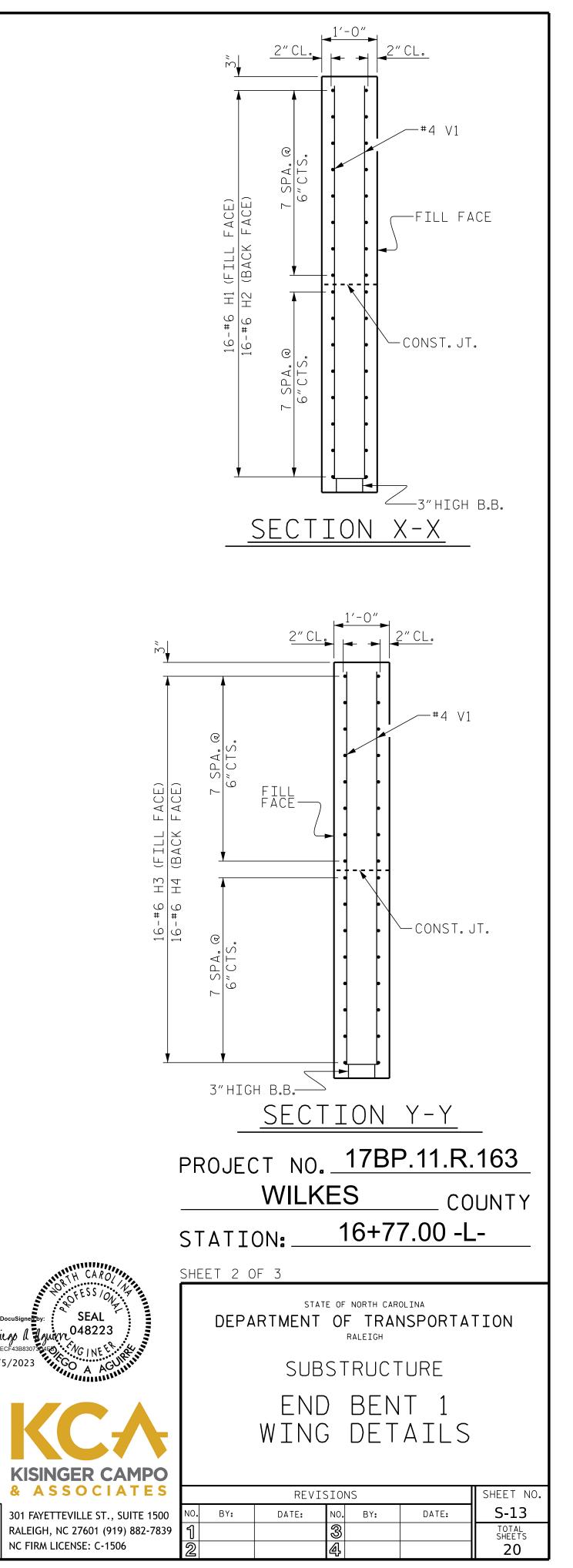
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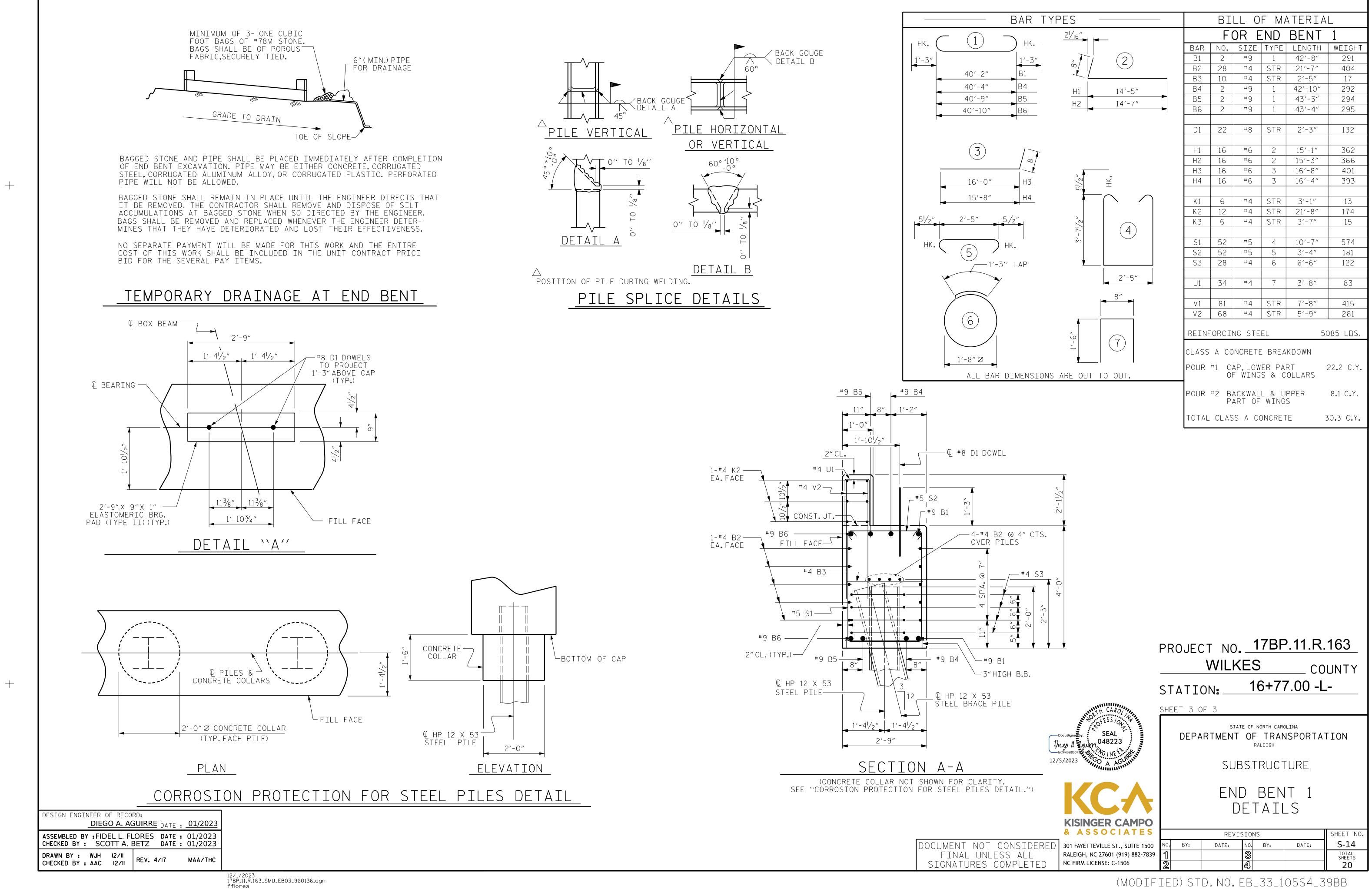


TOP OF PILE Elevations				
	1056.30			
2	1056.07			
3	1055.85			
4	1055.62			
5	1055.39			
6	1055.17			
	1054.94			

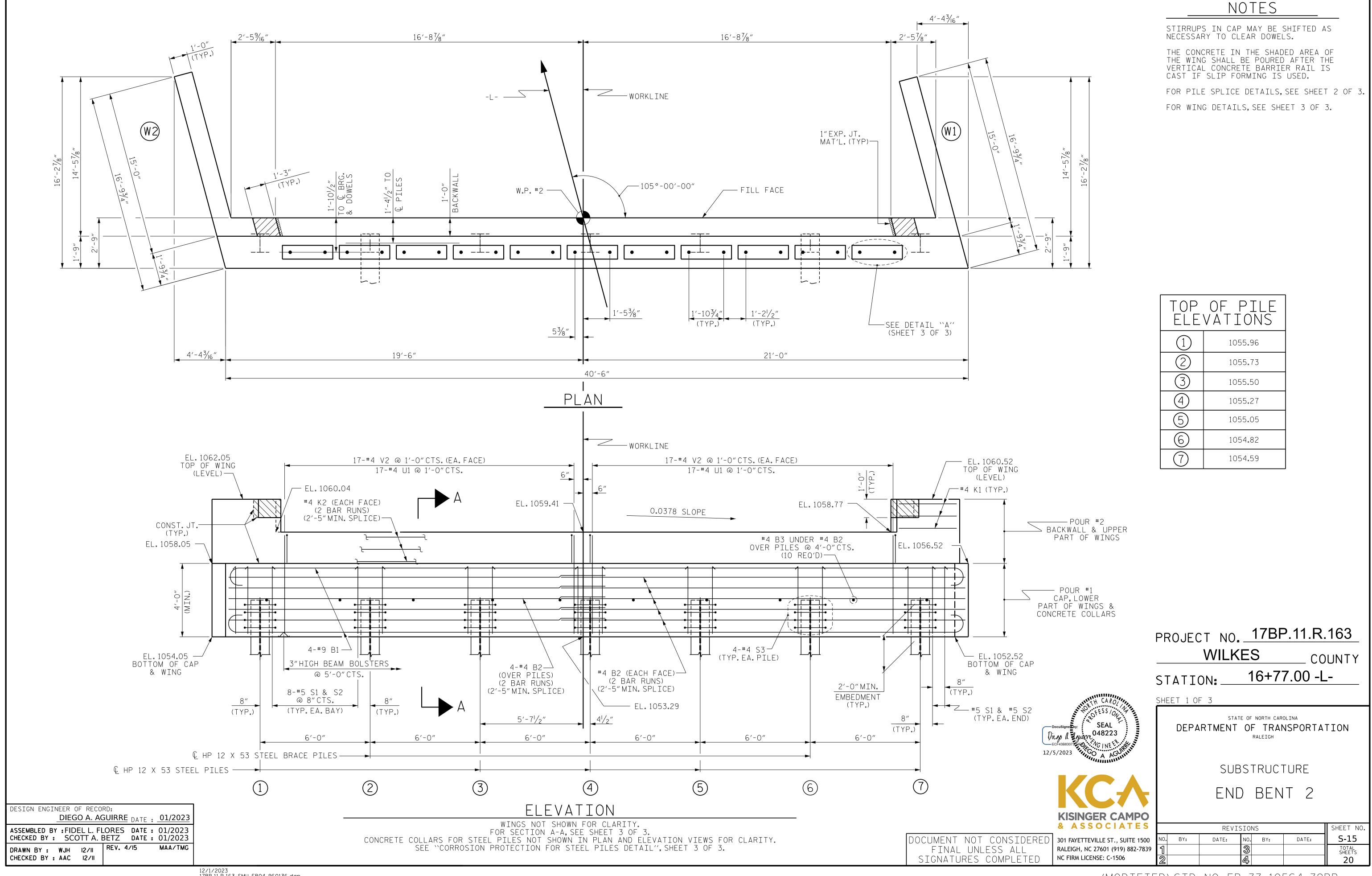


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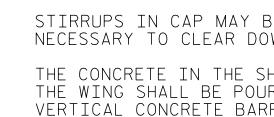


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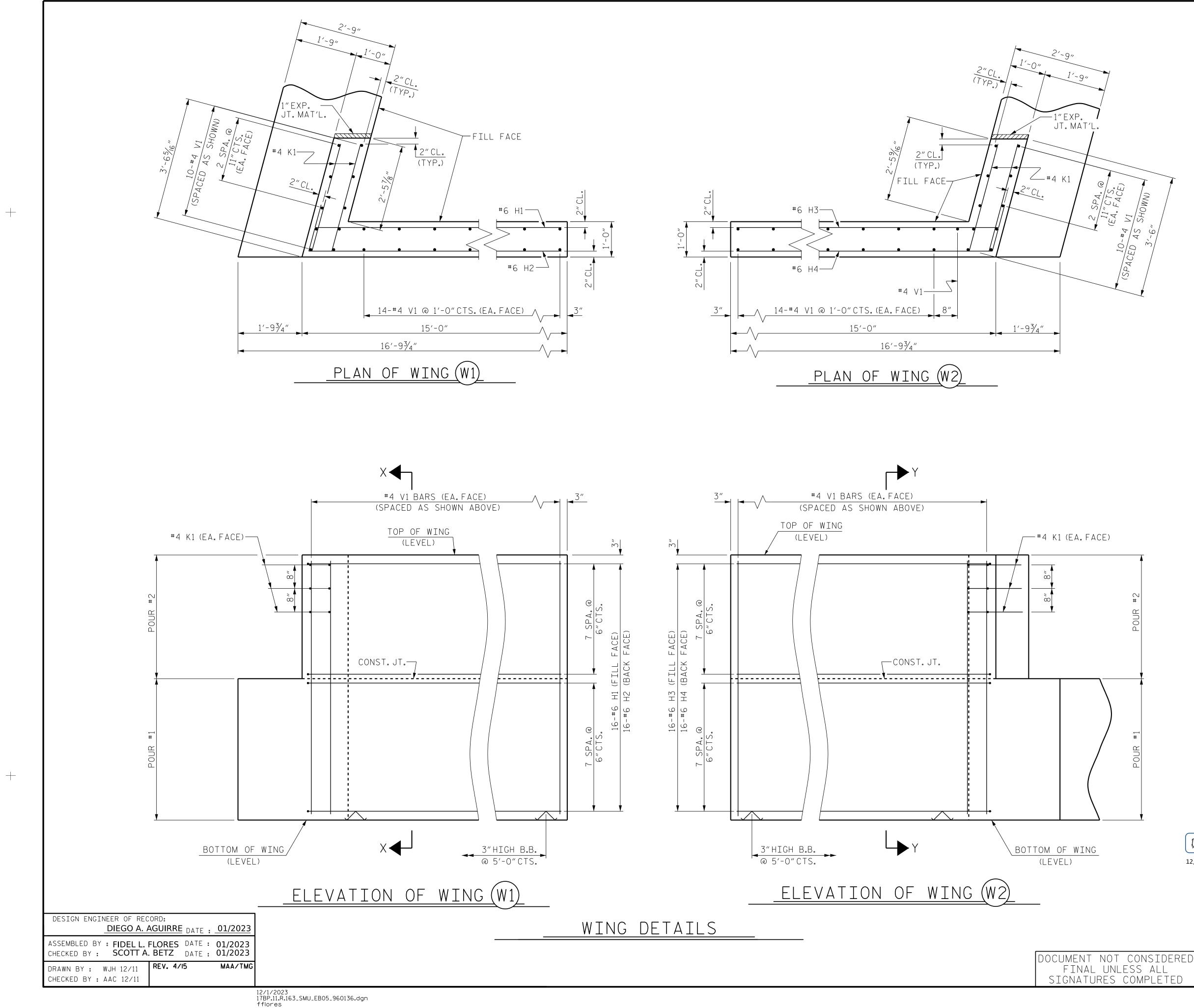


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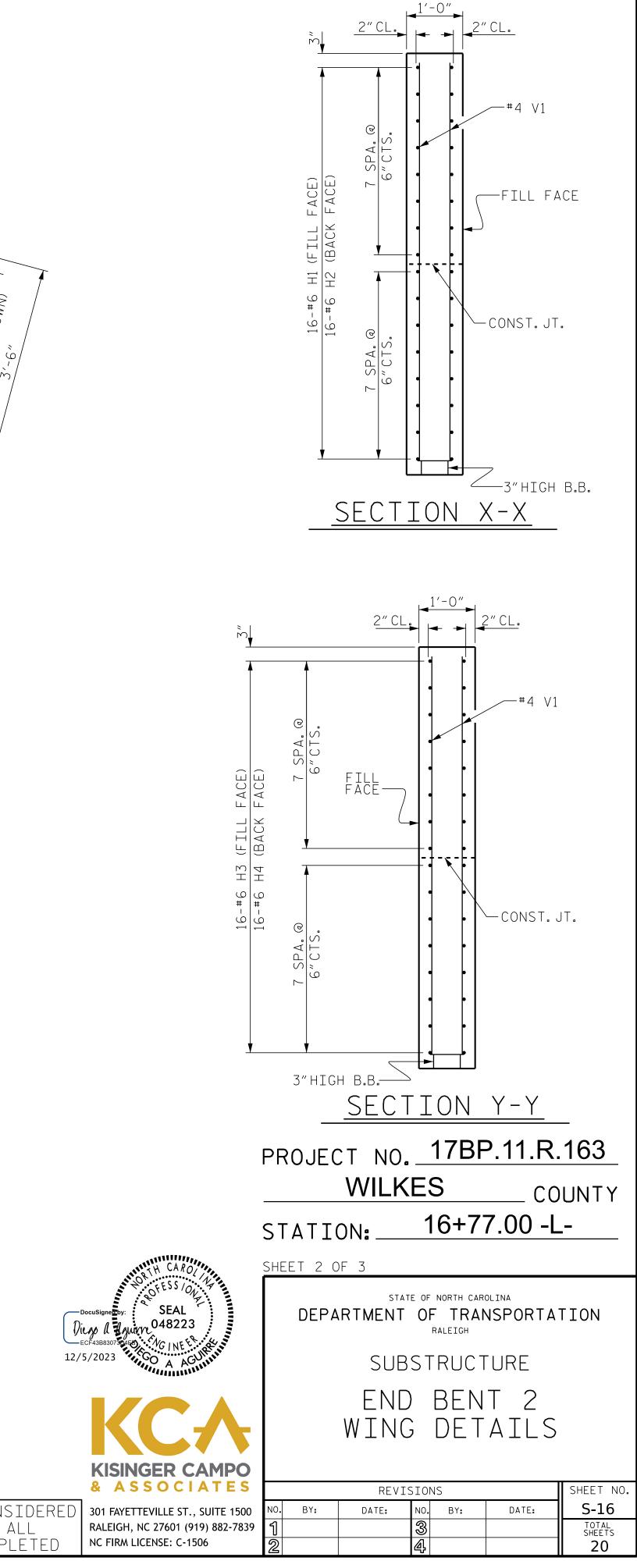
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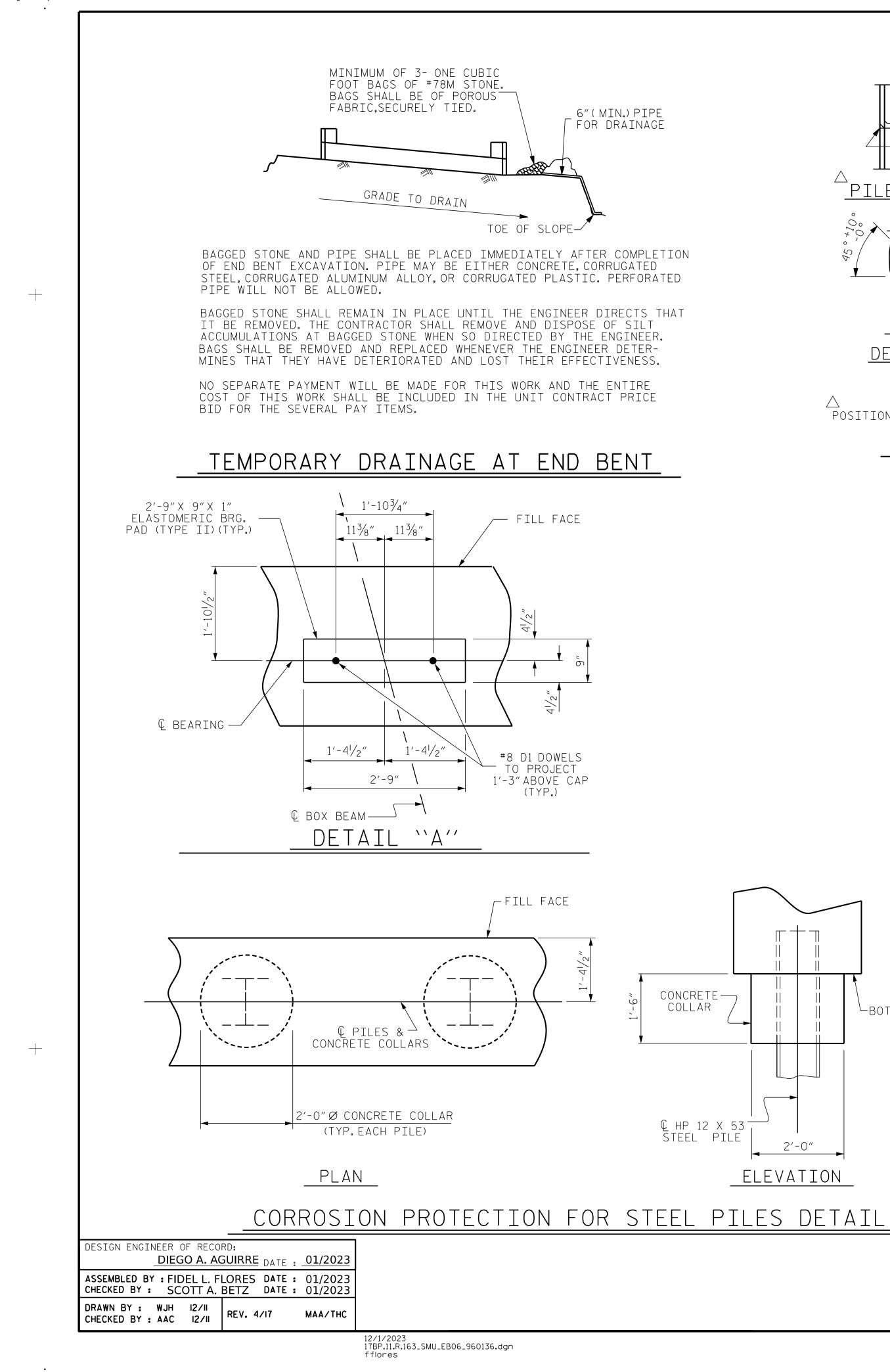
TOP ELE	OF PILE VATIONS
	1055.96
2	1055.73
3	1055.50
4	1055.27
5	1055.05
6	1054.82
7	1054.59

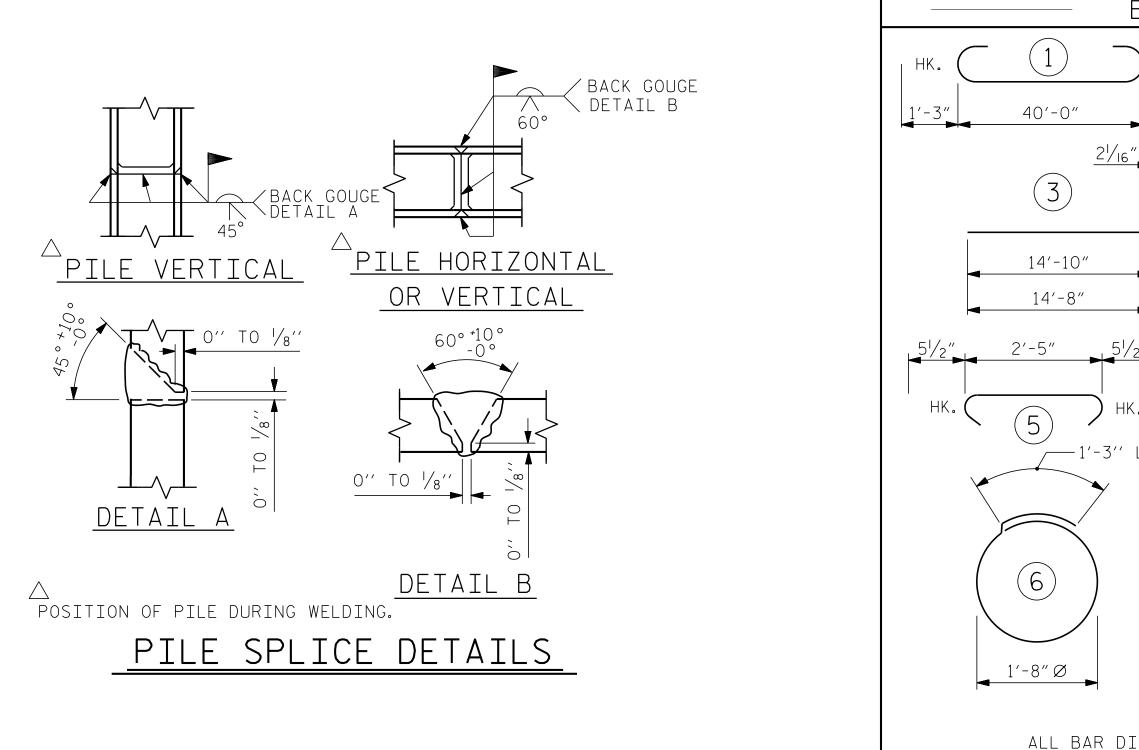


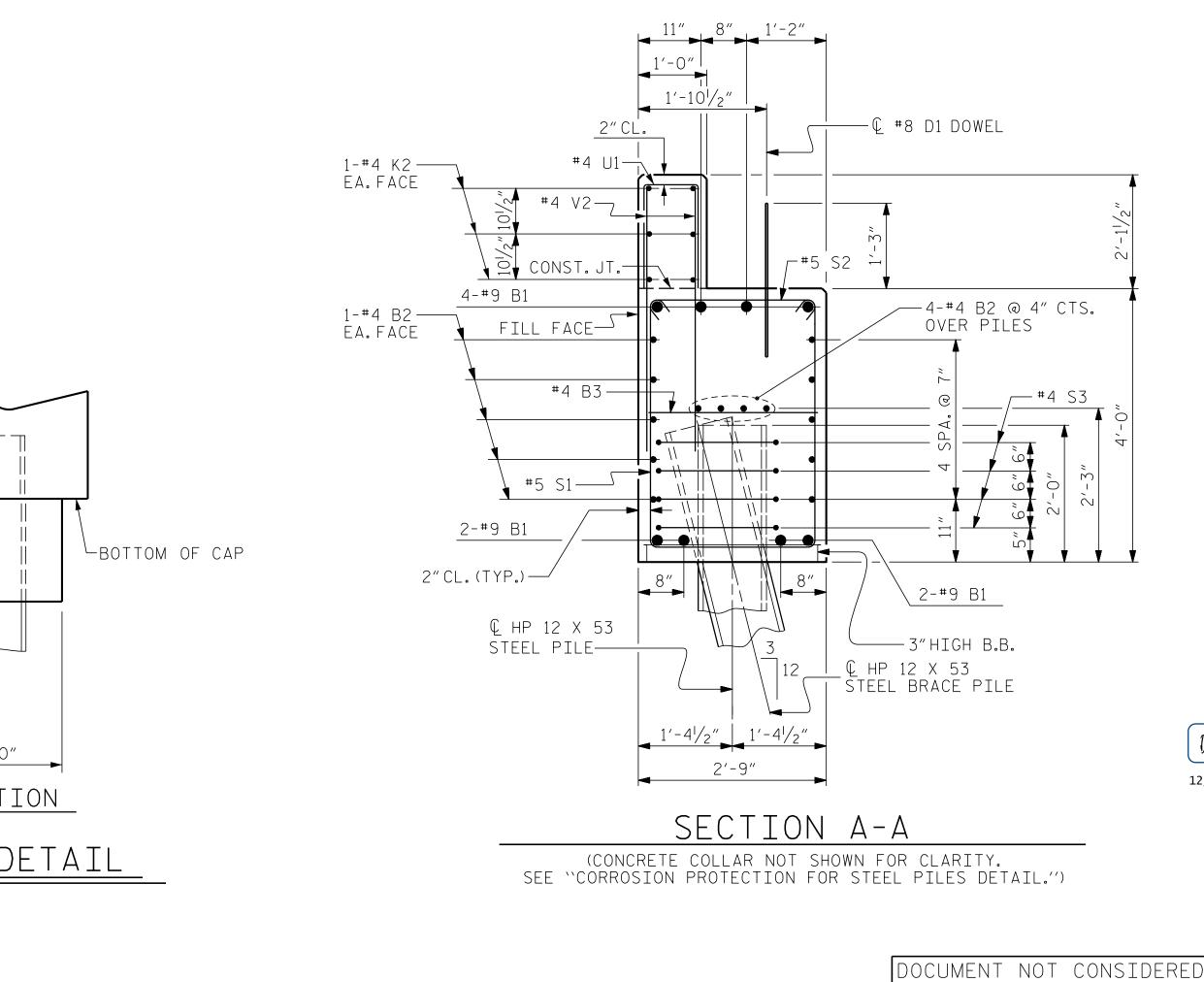
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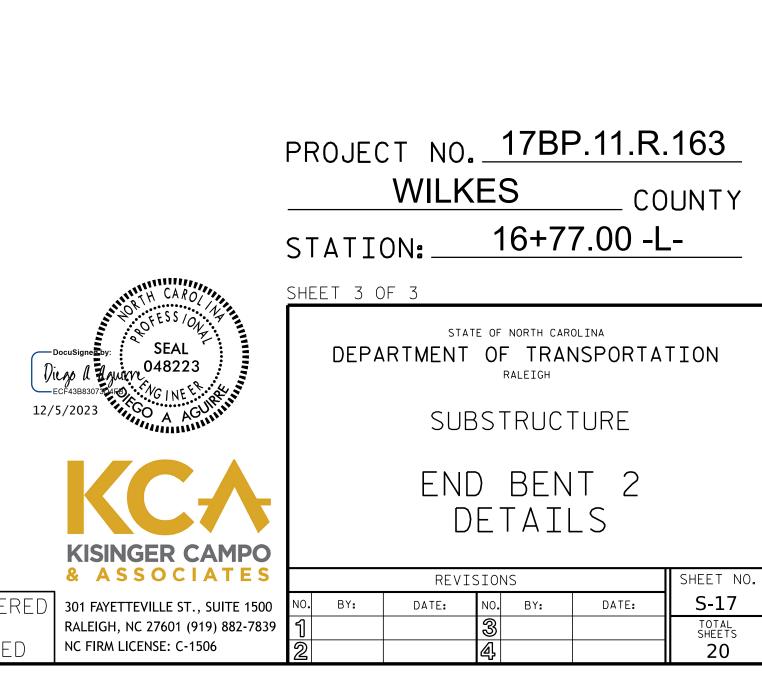


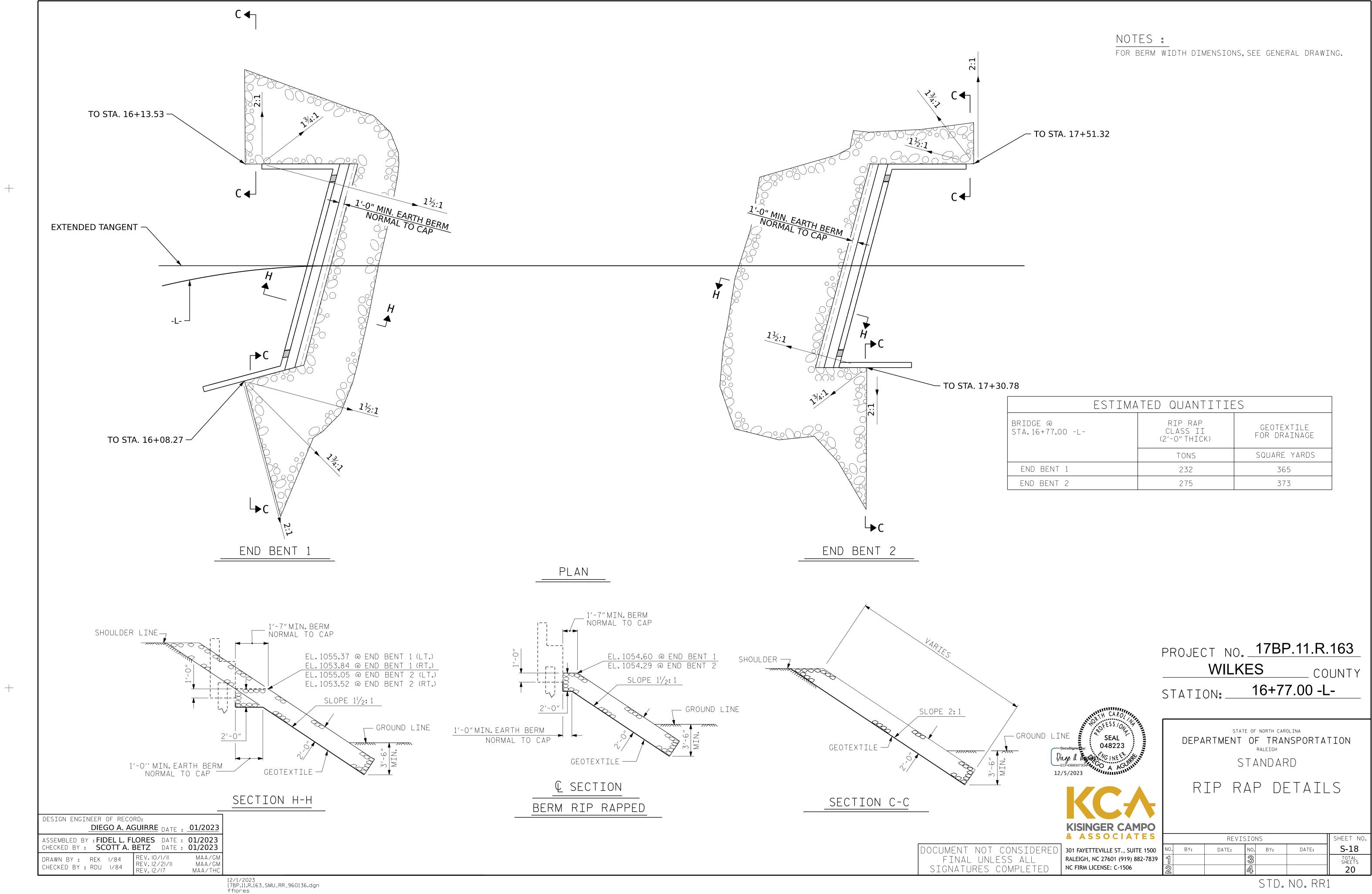




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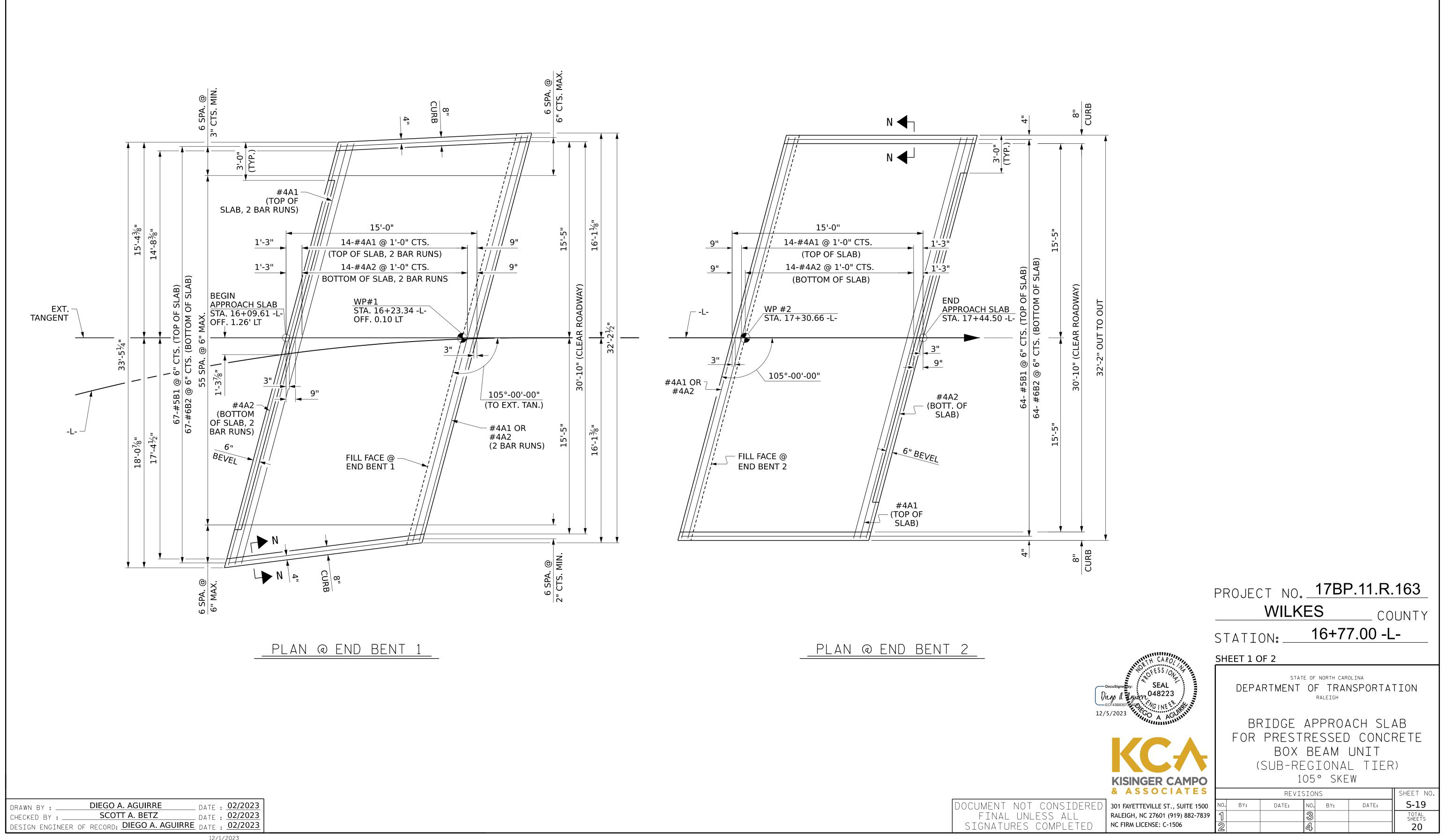
BAR TYP	PES		ΒI	LL O	FM/	ATERIA	
	<u>2<sup>1</sup>/<sub>16</sub>″</u>			END	BEN	NT 2	
) HK.		BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	$\tilde{\omega}$ (2)	B1	8	#9	1	42'-6"	1156
1'-3"		B2	28	#4	STR	21'-4"	399
		B3	10	#4	STR	2'-5"	16
	Н1 14'-5"	54			0 T D	01 7 11	170
	H2 14'-7"	D1	22	#8	STR	2'-3"	132
	× <b>–</b>	H1	16	#6	2	15'-1"	362
НЗ	HK.	H2	16	#6	2	15′-3″	366
H4		НЗ	16	#6	3	15′-6″	372
		H4	16	#6	3	15'-4"	368
/ <sub>2</sub> "					<u> </u>		
		K1	12	#4	STR	3'-1"	25
	<u>~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~</u>	K2	12	#4	STR	21'-4"	171
K.		S1	52	#5	4	10′-7″	574
LAP	♥	S2	52	#5	5	3'-4"	181
	2'-5"	S3	28	#4	6	6'-6"	122
		1 11	34	#4	7	3′-8″	83
	8″►	<u> </u>	54		1	5-0	0.5
		V1	77	#4	STR	7′-8″	394
		V2	68	#4	STR	5′-9″	261
		REINF	ORCIN	NG STE	EL	49	982 LBS.
	<u>▼</u>	CLASS	A CO	NCRETE	BREA	KDOWN	
		POUR #1 CAP,LOWER PART 21.9 C.Y. OF WINGS & COLLARS				21.9 C.Y.	
IMENSIUNS /	ARE OUT TO OUT.	POUR	#2 B4	ACKWAL Art of	L & UI	PPER	7.8 C.Y.
		TOTAL	CLAS	S A CO	NCRET	E 2	29.7 C.Y.





ESTIMATED QUANTITIES				
BRIDGE @ Sta.16+77.00 -L-	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE For drainage		
	TONS	SQUARE YARDS		
END BENT 1	232	365		
END BENT 2	275	373		

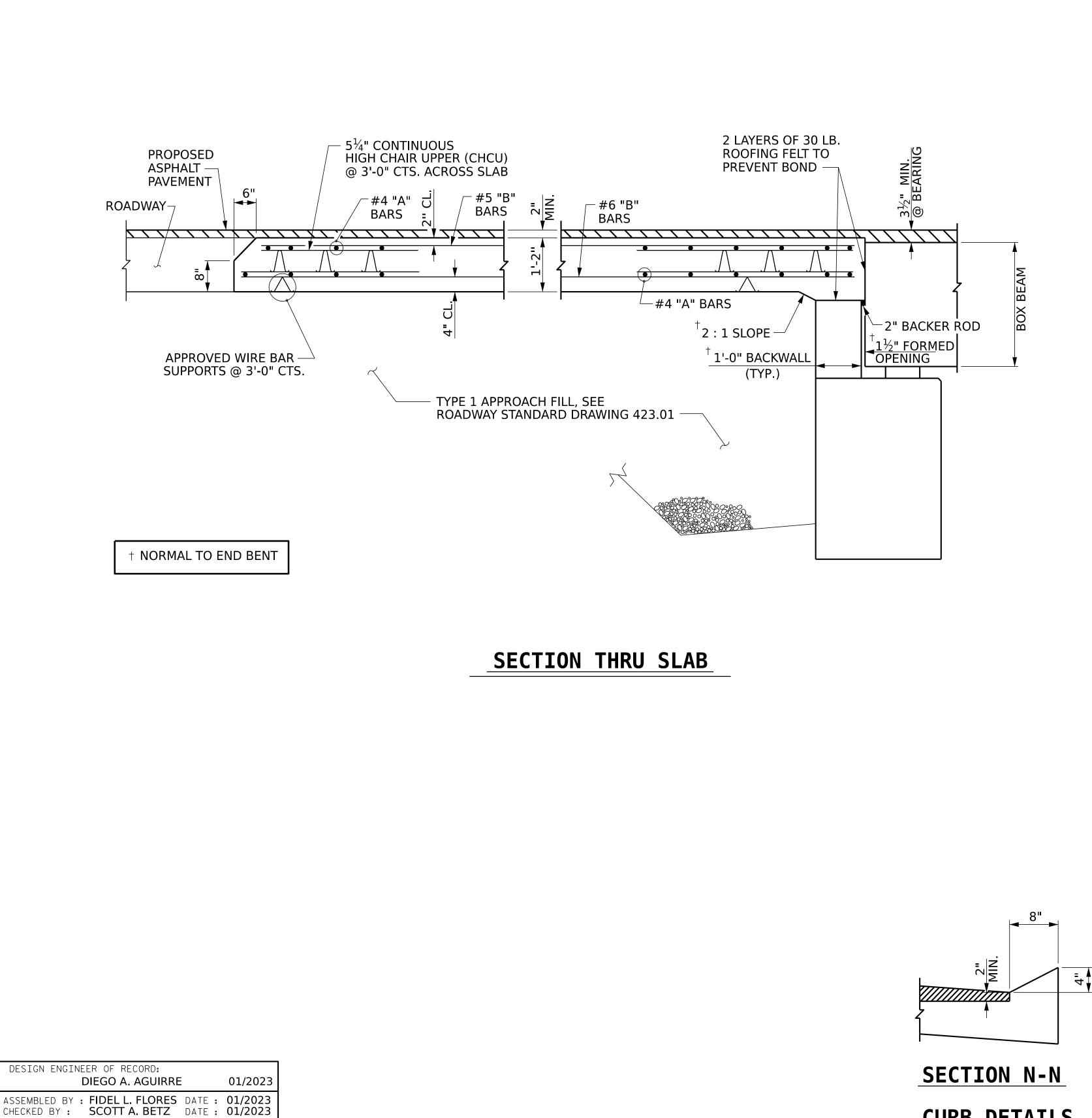
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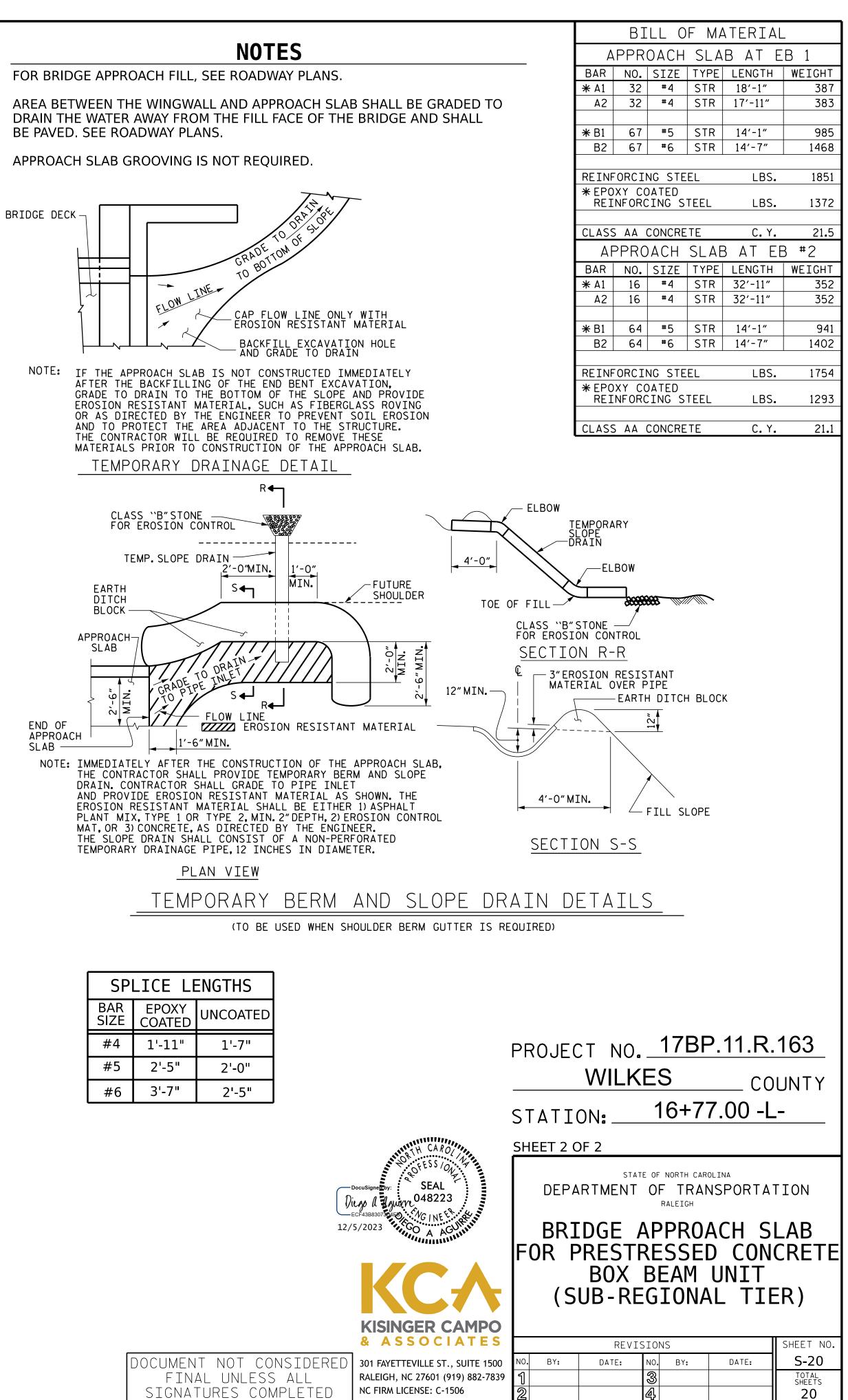
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REV. 12/17 MAA/THC REV. 06/19 BNB/THC REV. 07/23 BNB/SNM

DRAWN BY : FCJ 6/87

CHECKED BY : EGA 6/87



SPLICE LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	1'-11"	1'-7"
#5	2'-5"	2'-0"
#6	3'-7"	2'-5"

CURB DETAILS

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## DESIGN DATA: SPECIFICATIONS - - - - - - - - - - - - - A.A.S.H.T.O. (CURRENT) LIVE LOAD ---- SEE PLANS IMPACT ALLOWANCE - - - - - - - - - - - - - SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - - 20,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50W - - 27,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50 - - 27,000 LBS.PER SQ.IN. REINFORCING STEEL IN TENSION - GRADE 60 - - - 24.000 LBS.PER SQ.IN. CONCRETE IN COMPRESSION - - - - - - - - - - 1,200 LBS.PER SQ.IN. CONCRETE IN SHEAR - - - - - - - - - - - - SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS - - - 1,800 LBS.PER SQ.IN. COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER ---- 375 LBS.PER SQ.IN. EQUIVALENT FLUID PRESSURE OF EARTH - - - - 30 LBS.PER CU.FT. (MINIMUM)

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES. ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

## CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED  $\frac{3}{4}$ " with the following exceptions: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A  $\frac{1}{4}$  RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø studs specified on the plans. This substitution shall be made at THE RATE OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 1/8" & STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O". EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE. THE CONTRACTOR MAY, AT HIS OPTION.

# STANDARD NOTES

## ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

## HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

